_______ S2.02 (B) \ S2.03 ∕ WF2 __ _ _ 89'-0 1/2"

5

COLUN	IN SCHEDULE		
BASE PLATE SIZE	ANCHOR BOLTS	PEDESTAL SIZE AND REINFORCING	NOTES
12"x 1/2"x 1'-0"	(4)-3/4"Øx 8" LONG	16"x 16" PEDESTAL WITH (4)-#7 VERTICAL	PROVIDE PEDESTAL WHERE SHOWN ON PLAN

COLUMN SCHEDULE NOTES:

COLUMN SIZE

HSS6x6x1/4

MARK

- 1. FOR ORIENTATION OF COLUMNS, BASE PLATES, LEVEL PLATES, PEDESTALS, AND FOOTINGS - SEE PLAN
- 2. STRUCTURAL STEEL HSS SHAPES SHALL CONFORM TO ASTM A-500 GRADE B.
- STRUCTURAL STEEL PLATES SHALL CONFORM TO ASTM A-36 UNLESS NOTED
- 3. ALL ANCHOR BOLTS SHALL CONFORM TO ASTM F-1554, GRADE 55.
- 4. IN ADDITION TO DETAILED OUTER PEDESTAL TIES, PROVIDE ALL INTERIOR AND CROSS TIES AS REQUIRED BY ACI-318 LATEST EDITION.

FOOTING SCHEDULE				
MARK	FOOTING SIZE	REINFORCING	NOTES	
F30	3'-0" x 3'-0" x 12"	4#5 E.W. BOTTOM	PROVIDE PEDESTAL WHERE SHOWN ON PLAN	
F40	4'-0" x 4'-0" x 12"	5#5 E.W. BOTTOM	PROVIDE PEDESTAL WHERE SHOWN ON PLAN	
WF1	12" DEEP x 2'-0" CONTINUOUS	3#5 CONT. WITH #4 TIE BARS AT 24"o/c		
WF2	12" DEEP x 3'-0" CONTINUOUS	4#5 CONT. WITH #4 TIE BARS AT 24"o/c		

MASONRY WALL SCHEDULE			
FLOOR	BLOCK STRENGTH	VERTICAL REINFORCING	TENSION LAP SPLICE
LL TO GROUND	HIGH STRENGTH	#7 AT 8"	5'-3"
GROUND TO LEVEL 2	HIGH STRENGTH	#7 AT 16"	5'-3"
LEVEL 2 TO TRUSS BRG.	HIGH STRENGTH	#7 AT 24"	5'-3"

MASONRY WALL NOTES

- 1. FILL ALL REINFORCED MASONRY CELLS 100% SOLID WITH 3000 PSI GROUT.
- 2. LAP ALL #5 VERTICAL REINFORCING 2'-6" MINIMUM, #6 VERTICAL REINFORCING 3'-0" MINIMUM, AND #7 VERTICAL REINFORCING 3'-6" MINIMUM.
- 3. PROVIDE TENSION LAP SPLICE WHERE REQUIRED ON PLAN AND IN TYPICAL DETAILS.
- 4. PROVIDE FOOTING DOWELS AT WALL FOUNDATIONS SAME SIZE AND SPACING AS WALL VERTICAL REINFORCING. PROVIDE LAP SPLICE IN ACCORDANCE WITH
- 5. REINORCING SHOWN ON MASONRY RETAINING WALL DETAILS ON S2.02 SHALL SUPERCEDE AND REPLACE REINFORCING SHOWN IN SCHEDULE ABOVE.

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FOUNDATION AND LOWER LEVEL PLAN SCALE: 1/8" = 1'-0" 1. ELEVATION TOP OF STRUCTURAL SLAB ON GRADE = EL. -11.00 (U.N.O.)

2. STRUCTURAL SLAB SHALL BE 5" CONCRETE SLAB ON GRADE (fc = 3000 PSI) REINFORCED WITH 6" \times 6" - W2.9 / W2.9 WELDED WIRE FABRIC POURED OVER VAPOR BARRIER OVER 4" POROUS FILL. (U.N.O.)

3. EXTERIOR SLAB ON GRADE BY LANDSCAPE ARCHITECT. FOR ADDITIONAL INFORMATION SEE LANDSCAPE ARCHITECTURAL DRAWINGS.

4. ALL WALL FOOTINGS NOT SPECIFICALLY SHOWN ON PLAN SHALL BE 12" DEEP BY WALL WIDTH WITH 6" PROJECTION ON EACH SIDE OF WALL WITH 2#5 CONT. BARS AND #5 BARS AT 24"o.c.

5. ELEVATION BOTTOM OF FOOTING IS SHOWN ON PLAN.

ADDITIONAL INFORMATION AND REQUIREMENTS.

- 6. ASSUMED SOIL BEARING VALUE = 3000 PSF WAS USED IN DESIGN OF THE STRUCTURE. THIS VALUE SHALL BE FIELD VERIFIED BY A REGISTERED GEOTECHNICAL ENGINEER. SEE GEOTECHNICAL ENGINEERING REPORT PREPARED BY HERBST/BENSON & ASSOCIATES DATED JANUARY 6, 2017 FOR
- 7. WALL TYPES INDICATED ON PLAN THUS:
- REINFORCED MASONRY BLOCK WALLS (ASTM C-90)
- 8. STAIRS SHALL BE DESIGNED FOR 100 POUNDS PER SQUARE FOOT LIVE LOAD. SUBMIT SIGNED AND SEALED SHOP DRAWINGS TO ENGINEER FOR REVIEW. SEE ARCHITECTURAL DRAWINGS FOR RISER AND TREAD DIMENSIONS.
- 9. CONTRACTOR SHALL COORDINATE ALL DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL AND SITE DRAWINGS PRIOR TO CONSTRUCTION.

10.PROVIDE FOOTING IN SLAB ON GRADE AT ALL NON-LOAD BEARING BLOCK PARTITIONS PER DETAIL 9 ON S2.01. SEE ARCHITECTURAL DRAWINGS FOR

LOCATION OF ALL BLOCK PARTITIONS NOT SHOWN ON STRUCTURAL DRAWINGS. 11.SEE MECHANICAL DRAWINGS FOR EXACT SIZE AND LOCATION OF REQUIRED

EQUIPMENT PADS. SEE DETAIL 3 ON S2.02 FOR ADDITIONAL INFORMATION. 12.MASONRY WALLS SHALL BE 8" MASONRY (fm = 2500 PSI) REINFORCED PER MASONRY WALL SCHEDULE ON \$1.00.12. MASONRY WALLS SHALL BE 8" MASONRY

(f'm = 1500 PSI) REINFORCED PER MASONRY WALL SCHEDULE ON S1.00.

ARCHITECT

11

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REVISIONS REV. # DESCRIPTION

KEY PLAN

INTERFAITH CENTER



ARCHITECTS + PLANNERS

NOT FOR CONSTRUCTION

DRAWING IN	FORMATION
ISSUE DATE:	04/28/17
SCALE:	As indicated
JOB NO.:	16072
DRAWN BY:	CRS

PROJECT DESIGN PHASE

50% CD SET

DRAWING NAME FOUNDATION AND LOWER

LEVEL PLAN

DRAWING NUMBER

S1.00

No.

16"o/c MAX.

ML1+ML2

ML1+ML2

16'-0 7/8"

S2.02

GROUND LEVEL PL

SCALE: 1/8" = 1'-0"

1. ELEVATION TOP OF STRUCTURAL WOOD FRAMED FLOOR SHALL BE 0.00.

2. STRUCTURAL FLOOR SHALL BE 3/4" GYPSUM TOPPING (TYPICAL, REFER TO ARCHITECTURAL DRAWINGS FOR OTHER THICKNESSES) OVER 1/4" ACOUSTIMAT OVER 23/32" TONGUE AND GROOVE EXPOSURE 1 APA GROUP 1 RATED STURD-I-FLOOR NAILED WITH 8d COMMON NAILS AT 6"o/c ALONG PANEL EDGES AND 12"o/c ALONG INTERMEDIATE SUPPORTS. AS AN ALTERNATE, CONTRACTOR MAY GLUE AND NAIL FLOOR SHEATHING TO FLOOR TRUSSES WITH 6d RING OR SCREW SHANK NAILS AT 6"o/c ALONG PANEL EDGES AND 12"o/c ALONG INTERMEDIATE SUPPORTS.

3. ELEVATION TOP OF STEEL BEAMS SHOWN ON PLAN = -0.14.

4. WALL TYPES ARE SHOWN ON PLAN, THUS:

WOOD STUD WALLS

REINFORCED MASONRY BLOCK WALLS (ASTM C-90)

5. CONTRACTOR SHALL COORDINATE ALL DIMENSIONS AND ELEVATIONS WITH ARCHITECT PRIOR TO CONSTRUCTION.

6. ALL WOOD STUDS SHALL BE NO. 1 / NO. 2 SPRUCE PINE FIR (UNO). APARTMENT WOOD STUD BEARING / SHEAR WALLS BELOW THIRD FLOOR SHALL BE COMPRISED OF THE FOLLOWING, UNLESS NOTED OTHERWISE:

LOCATION STUD SIZE / SPACING
EXTERIOR 2x8 AT 16"o/c
STAIR SHAFT 2x6 AT 16"o/c
ELEVATOR SHAFT 2x6 AT 16"o/c

ELEVATOR SHAFT 2x6 AT 16"o/c NON-BEARING 2x4 AT 24"o/c (UNO)

WOOD STUD BEARING WALLS SHOWN ON PLAN SHALL BE BLOCKED AT 4'-0"o/c

7. WOOD TRUSS LAYOUT AS SHOWN ON PLAN IS DIAGRAMMATIC. ALIGN TRUSSES WITH STUDS BELOW WHERE POSSIBLE. COORDINATE ALL TRUSS LOCATIONS WITH MECHANICAL FLOOR PARAMETERS. PROVIDE DOUBLE JOISTS AND HEADERS AT ALL OPENINGS WIDER THAN TRUSS SPACING. CONNECT HEADER TO DOUBLE TRUSS WITH JOIST HANGERS.

8. PRE-ENGINEERED WOOD FLOOR TRUSSES NOTED, THUS:

PWT WOOD TRUSSES AT DEPTH AND SPACING NOTED ON PLAN

9. PRE-ENGINEERED TRUSSES SHALL BE DESIGNED FOR THE FOLLOWING MINIMUM SUPERIMPOSED LIVE AND DEAD LOADS BEYOND THE SELF WEIGHT OF TRUSSES:

 FLOOR TYPE
 TOP CHORD
 BOTTOM CHORD

 DEAD
 LIVE
 DEAD
 LIVE

 COMMON AREAS
 15 PSF
 100 PSF
 5 PSF
 0 PSF

WOOD FLOOR TRUSS DEFLECTION SHALL NOT EXCEED L/480 FOR LIVE LOAD AND L/360 FOR TOTAL LOAD.

10.FOR WOOD BEAM AND LINTEL SCHEDULES, REFER TO SHEET S1.01. ALL WOOD BEAMS NOTED, THUS: 'B0' ARE TO BE FLUSH FRAMED WITH FLOOR. ALL WOOD LINTELS NOTED, THUS: 'L0' ARE TO BE ELEVATED AT HEAD OF OPENING.

11.STAIRS SHALL BE DESIGNED FOR 100 POUNDS PER SQUARE FOOT LIVE LOAD. SUBMIT SIGNED AND SEALED SHOP DRAWINGS OF STAIR FRAMING AND HANDRAILS PREPARED BY A REGISTERED PROFESSIONAL ENGINEER FROM THE STATE OF MARYLAND FOR REVIEW.

12.MASONRY WALLS SHALL BE 8" MASONRY (fm = 2500 PSI) REINFORCED PER MASONRY WALL SCHEDULE ON S1.00.

13.REFER TO ARCHITECTURAL AND MECHANICAL DRAWINGS FOR LOCATIONS AND REQUIREMENTS AT LOUVERED OPENINGS BELOW EXTERIOR WINDOWS. FOR FRAMING MODIFICATIONS AT LOUVERED OPENINGS, SEE DETAIL

14.STRUCTURAL STEEL WIDE FLANGE BEAMS SHALL CONFORM TO ASTM A-992, GRADE 50. HOLLOW STRUCTURAL STEEL TUBE SHALL CONFORM TO ASTM A500, GRADE B (Fy=46 KSI). STRUCTURAL STEEL PLATES, ANGLES AND CHANNELS SHALL CONFORM TO ASTM A36.

15.ALL EXPOSED STRUCTURAL STEEL SHALL BE HOT-DIPPED GALVANIZED.

16.STAIRS SHALL BE DESIGNED FOR 100 POUNDS PER SQUARE FOOT LIVE LOAD. SUBMIT SIGNED AND SEALED SHOP DRAWINGS TO ENGINEER FOR REVIEW. STAIRS TO BE METAL PAN. SEE ARCHITECTURAL DRAWINGS FOR RISERS AND TREAD DIMENSIONS. ALL STAIR CHANNELS BEARING ON MASONRY WALLS SHALL REQUIRE 6"x 6"x 1/4" BEARING PLATES AND 1/2"Ø x 8" LONG ANCHOR BOLT WITH 2" HOOK.

17.PROVIDE DOUBLE 8" WIDE x 8" DEEP CONT. BOND BEAM AROUND PERIMETER OF ELEVATOR AND STAIR WITH 2#5 CONT. BOTTOM REINFORCING EACH COURSE AND FILLED SOLID WITH 3000 PSI GROUT AT EACH FLOOR AND ROOF ELEVATION. PROVIDE 90 DEGREE HOOK BARS AT CORNERS.

18.EXTERIOR SLAB ON GRADE BY LANDSCAPE ARCHITECT. FOR ADDITIONAL INFORMATION SEE LANDSCAPE ARCHITECTURAL DRAWINGS.

WELDED WIRE FABRIC POURED OVER VAPOR BARRIER OVER 4" POROUS FILL. (U.N.O.)

FOR ADDITIONAL INFORMATION.

20.STRUCTURAL SLAB SHALL BE 5" CONCRETE SLAB ON GRADE (fc = 3000 PSI) REINFORCED WITH 6" x 6" - W2.9 / W2.9

	LINTEL SCHEDULE				
MARK	MEMBER	TYPE	REMARKS		
ML1	(2)-6"x 3 1/2"x 3/8" ANGLE (LLV)		8" MIN. BEARING AT EACH END		
ML2	5"x 5"x 3/4" ANGLE		8" MIN. BEARING AT EACH END		
ML3	W8x28 WITH 16" WIDE x 3/4" THICK CONT. PLATE		16" MIN. BEARING AT EACH END		
L1	(2)-2x6 + FULL-DEPTH BLOCKING AT MIDPOINT		PROVIDE (2)-2x8 JACK STUDS EACH END		
L2	(2)-2x8 + FULL-DEPTH BLOCKING AT MIDPOINT		PROVIDE (2)-2x8 JACK STUDS EACH END		
L3	(4)-2x10 + 1/2" PLYWOOD PLATE + 3/4" PLYWOOD PLATE		PROVIDE (2)-2x8 JACK STUDS EACH END		
L4	(4)-2x12 + 1/2" PLYWOOD PLATE + 3/4" PLYWOOD PLATE		PROVIDE (2)-2x8 JACK STUDS EACH END		

LINTEL NOTES

1. REFER TO ARCHITECTURAL DRAWINGS FOR EXACT SIZE AND LOCATION OF WALL OPENINGS.

 SECURE MULTIPLE SAWN MEMBERS TOGETHER WITH (2) ROWS OF 16d NAILS AT 12"0/c FOR MEMBERS UP TO 12" DEEP AND (3) ROWS OF 16d NAILS FOR MEMBERS GREATER THAN 12" DEEP (UNLESS NOTED OTHERWISE). USE GALVANIZED NAILS FOR ALL EXTERIOR LINTELS.

3. ALL WOOD LINTELS EXPOSED TO THE WEATHER SHALL BE PRESSURE TREATED.

4. STRUCTURAL WOOD LINTELS SHALL BE NO.1/NO.2 SPRUCE PINE FIR WITH THE FOLLOWING MINIMUM PROPERTIES:

Fb = 875 PSI Fc = 425 PSI Fv = 135 PSI Ft = 450 PSI Fc|| = 1,150 PSI E = 1,400,000 PSI

6. CONTRACTOR HAS OPTION TO PROVIDE ADDITIONAL WOOD JOISTS AND PLYWOOD PLATES TO WOOD LINTELS AS NOTED TO ACHEIVE DESIRED THICKNESS.

7. SEE PLAN FOR END POST OR END JACK STUDS AND PROVIDE (2) FULL HEIGHT KING STUDS EACH END OF LINTEL.
SECURE MULTIPLE JAMB STUDS TOGETHER WITH 16d NAILS AT 8"0/c STAGGERED, EACH PLY

SECURE MULTIPLE JAMB STUDS TOGETHER WITH 16d NAILS AT 8"o/c STAGGERED, EACH PLY.

8. FOR ADDITIONAL INFORMATION AT TYPICAL WOOD LINTEL FRAMING, REFER TO DETAILS (\$3.02)

9. ALL STRUCTURAL STEEL EXPOSED TO THE WEATHER OR THAT ARE IN CONTACT WITH MASONRY SHALL BE

HOT-DIPPED GALVANIZED IN ACCORDANCE WITH SPECIFICATION SECTION 05100.

10.PROVIDE 8" MINIMUM BEARING AT EACH END OF ANGLE LINTEL.

11.WOOD POST AND BUILT-UP WOOD POST TO BEAM CONNECTIONS: PROVIDE CC COLUMN CAP SERIES SIZED ACCORDING TO BEAM WIDTH AND BEARING CONDITION. PROVIDE 5/8" DIA. BOLTS AND WOOD SHIM PLATES

AS REQUIRED, SEE DETAIL 11 S3.02

GOUCHEF —college—

PROJECT INFORMATION

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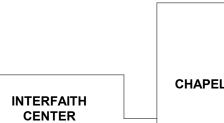
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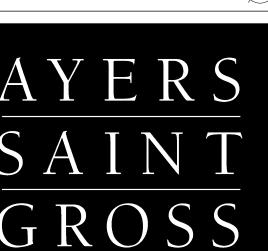
REVISIONS

REV. # DESCRIPTION DATE

KEY PLAN



RFAITH NTER



ARCHITECTS + PLANNERS

NOT FOR CONSTRUCTION

	DRAWING INFORMATION	
ISSUE DAT	E:	04/28/17
SCALE:		As indicated
JOB NO.:		16072
DRAWN BY	/ :	CRS
	PROJECT DESIGN PHASE	

PROJECT DESIGN FITASE

50% CD SET

DRAWING NAME

GROUND LEVEL FRAMING

DRAWING NUMBER



SECOND FLOOR FRAMING PLAN

1. ELEVATION TOP OF FRAMED STRUCTURAL SLAB AT HAMBRO FLOOR = EL. 11.33 (U.N.O.) 2. STRUCTURAL FLOOR SHALL BE 3/4" GYPSUM TOPPING (TYPICAL, REFER TO ARCHITECTURAL DRAWINGS FOR OTHER THICKNESSES) OVER 1/4" ACOUSTIMAT OVER 23/32" TONGUE AND GROOVE EXPOSURE 1 APA GROUP 1 RATED STURD-I-FLOOR NAILED WITH 8d COMMON NAILS AT 6"o/c ALONG PANEL EDGES AND 12"o/c ALONG INTERMEDIATE SUPPORTS. AS AN ALTERNATE, CONTRACTOR MAY GLUE AND NAIL FLOOR SHEATHING TO FLOOR TRUSSES WITH 6d RING OR SCREW SHANK NAILS AT 6"o/c ALONG PANEL EDGES AND 12"o/c ALONG

WOOD STUD WALLS

6. ALL WOOD STUDS SHALL BE NO. 1 / NO. 2 SPRUCE PINE FIR (UNO). APARTMENT WOOD STUD BEARING / SHEAR WALLS BELOW THIRD FLOOR SHALL BE COMPRISED OF THE FOLLOWING, UNLESS NOTED OTHERWISE:

STUD SIZE / SPACING 2x8 AT 16"o/c STAIR SHAFT 2x6 AT 16"o/c ELEVATOR SHAFT 2x6 AT 16"o/c

POSSIBLE. COORDINATE ALL TRUSS LOCATIONS WITH MECHANICAL FLOOR PARAMETERS. PROVIDE DOUBLE

9. PRE-ENGINEERED TRUSSES SHALL BE DESIGNED FOR THE FOLLOWING MINIMUM SUPERIMPOSED LIVE AND DEAD

TOP CHORD BOTTOM CHORD FLOOR TYPE DEAD LIVE DEAD LIVE

10.FOR WOOD BEAM AND LINTEL SCHEDULES, REFER TO SHEET S1.01. ALL WOOD BEAMS NOTED, THUS: 'B0' ARE TO BE

11.STAIRS SHALL BE DESIGNED FOR 100 POUNDS PER SQUARE FOOT LIVE LOAD. SUBMIT SIGNED AND SEALED SHOP DRAWINGS OF STAIR FRAMING AND HANDRAILS PREPARED BY A REGISTERED PROFESSIONAL ENGINEER FROM THE

13.REFER TO ARCHITECTURAL AND MECHANICAL DRAWINGS FOR LOCATIONS AND REQUIREMENTS AT LOUVERED

14.STRUCTURAL STEEL WIDE FLANGE BEAMS SHALL CONFORM TO ASTM A-992, GRADE 50. HOLLOW STRUCTURAL STEEL TUBE SHALL CONFORM TO ASTM A500, GRADE B (Fy=46 KSI). STRUCTURAL STEEL PLATES, ANGLES AND CHANNELS SHALL CONFORM TO ASTM A36.

15.ALL EXPOSED STRUCTURAL STEEL SHALL BE HOT-DIPPED GALVANIZED.

PLATES AND 1/2"Ø x 8" LONG ANCHOR BOLT WITH 2" HOOK. 17.PROVIDE DOUBLE 8" WIDE x 8" DEEP CONT. BOND BEAM AROUND PERIMETER OF ELEVATOR AND STAIR WITH 2#5 CONT. BOTTOM REINFORCING EACH COURSE AND FILLED SOLID WITH 3000 PSI GROUT AT EACH FLOOR AND



SCALE: 1/8" = 1'-0"

11

INTERMEDIATE SUPPORTS.

3. ELEVATION TOP OF STEEL BEAMS SHOWN ON PLAN = 11.19.

4. WALL TYPES ARE SHOWN ON PLAN, THUS:

REINFORCED MASONRY BLOCK WALLS (ASTM C-90)

5. CONTRACTOR SHALL COORDINATE ALL DIMENSIONS AND ELEVATIONS WITH ARCHITECT PRIOR TO CONSTRUCTION.

NON-BEARING 2x4 AT 24"o/c (UNO) WOOD STUD BEARING WALLS SHOWN ON PLAN SHALL BE BLOCKED AT 4'-0"o/c

7. WOOD TRUSS LAYOUT AS SHOWN ON PLAN IS DIAGRAMMATIC. ALIGN TRUSSES WITH STUDS BELOW WHERE JOISTS AND HEADERS AT ALL OPENINGS WIDER THAN TRUSS SPACING. CONNECT HEADER TO DOUBLE TRUSS WITH JOIST HANGERS.

8. PRE-ENGINEERED WOOD FLOOR TRUSSES NOTED, THUS:

PWT WOOD TRUSSES AT DEPTH AND SPACING NOTED ON PLAN

LOADS BEYOND THE SELF WEIGHT OF TRUSSES:

WOOD FLOOR TRUSS DEFLECTION SHALL NOT EXCEED L/480 FOR LIVE LOAD AND L/360 FOR TOTAL LOAD.

FLUSH FRAMED WITH FLOOR. ALL WOOD LINTELS NOTED, THUS: 'LO' ARE TO BE ELEVATED AT HEAD OF OPENING.

STATE OF MARYLAND FOR REVIEW.

12.MASONRY WALLS SHALL BE 8" MASONRY (fm = 2500 PSI) REINFORCED PER MASONRY WALL SCHEDULE ON S1.00.

OPENINGS BELOW EXTERIOR WINDOWS. FOR FRAMING MODIFICATIONS AT LOUVERED OPENINGS, SEE DETAIL

16.STAIRS SHALL BE DESIGNED FOR 100 POUNDS PER SQUARE FOOT LIVE LOAD. SUBMIT SIGNED AND SEALED SHOP DRAWINGS TO ENGINEER FOR REVIEW. STAIRS TO BE METAL PAN. SEE ARCHITECTURAL DRAWINGS FOR RISERS AND TREAD DIMENSIONS. ALL STAIR CHANNELS BEARING ON MASONRY WALLS SHALL REQUIRE 6"x 6"x 1/4" BEARING

ROOF ELEVATION. PROVIDE 90 DEGREE HOOK BARS AT CORNERS.

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REVISIONS DESCRIPTION

KEY PLAN





ARCHITECTS + PLANNERS

GROSS

NOT FOR CONSTRUCTION

ISSUE DATE: JOB NO.: DRAWN BY:

PROJECT DESIGN PHASE

50% CD SET

DRAWING NAME

LEVEL 2 FRAMING PLAN

DRAWING NUMBER **S1.02**

PWRT AT 24"o/c MAX. 2x12 AT 12"0/c MAX. S4.02 ML1+ML2 21'-11 7/8" 10'-0"

HIGH ROOF FRAMING PLAN

ELEVATION TOP OF STRUCTURAL WOOD FRAMED ROOF VARIES. SEE ARCHITECTURAL DRAWINGS

SCALE: 1/8" = 1'-0"

3

FOR CONTROL POINTS. SLOPE TOP CHORD OF WOOD TRUSSES TO ACCOMMODATE ROOF PITCH. 2. STRUCTURAL ROOF DECK OVER WOOD TRUSSES SHALL BE 19/32" EXTERIOR 24/16 EXPOSURE 1 APA RATED

- PLYWOOD FASTENED TO ROOF TRUSSES. SEE SHEET S1.03 FOR FASTENING SCHEDULE.
- 3. WALL TYPES ARE SHOWN ON PLAN, THUS:
- WOOD STUD WALLS REINFORCED MASONRY BLOCK WALLS (ASTM C-90)

4. CONTRACTOR SHALL COORDINATE ALL DIMENSIONS AND ELEVATIONS WITH ARCHITECT PRIOR TO CONSTRUCTION.

5. ALL WOOD STUDS SHALL BE NO. 1 / NO. 2 SPRUCE PINE FIR AND SHALL BE COMPRISED OF THE FOLLOWING, UNLESS NOTED OTHERWISE:

STUD SIZE / SPACING 2x8 AT 16"o/c

NON-BEARING 2x4 AT 24"o/c (UNO)

WOOD STUD BEARING WALLS SHALL BE BLOCKED AT 4'-0"o/c MAX.

6. WOOD TRUSS LAYOUT AS SHOWN ON PLAN IS DIAGRAMMATIC. ALIGN TRUSSES WITH STUDS BELOW WHERE POSSIBLE. COORDINATE ALL TRUSS LOCATIONS WITH MECHANICAL FLOOR PARAMETERS. PROVIDE DOUBLE JOISTS AND HEADERS AT ALL OPENINGS WIDER THAN TRUSS SPACING. CONNECT HEADER TO DOUBLE TRUSS WITH JOIST HANGERS.

PRE-ENGINEERED WOOD ROOF TRUSSES NOTED, THUS:

PWRT AT WOOD ROOF TRUSSES AT 24"o/c MAX.

7. PRE-ENGINEERED TRUSSES SHALL BE DESIGNED FOR THE FOLLOWING MINIMUM SUPERIMPOSED LIVE AND DEAD LOADS BEYOND THE SELF WEIGHT OF TRUSSES:

ROOF TYPE	TOP CHORD		BOTTOM CHORD	
NOOI TITE	DEAD	LIVE	DEAD	LIVE
TYPICAL ROOF	15 PSF	30 PSF	10 PSF	0 PSF

- PRE-ENGINEERED WOOD ROOF TRUSS DEFLECTION SHALL NOT EXCEED L/360 OR 1" MAXIMUM FOR LIVE LOAD AND L/240 FOR TOTAL LOAD.
- 8. FOR WOOD BEAM AND LINTEL SCHEDULES, REFER TO SHEET S1.01. ALL WOOD BEAMS NOTED, THUS: 'B0' ARE TO BE FLUSH FRAMED WITH TRUSS BEARING CONDITIONS. ALL WOOD LINTELS NOTED, THUS: 'LO' ARE TO BE ELEVATED AT
- 9. STAIRS SHALL BE DESIGNED FOR 100 POUNDS PER SQUARE FOOT LIVE LOAD. SUBMIT SIGNED AND SEALED SHOP DRAWINGS OF STAIR FRAMING AND HANDRAILS PREPARED BY A REGISTERED PROFESSIONAL ENGINEER FROM THE

STATE OF MARYLAND FOR REVIEW. 10.MASONRY WALLS SHALL BE 8" MASONRY (fm = 2500 PSI) REINFORCED PER MASONRY WALL SCHEDULE ON S1.00.

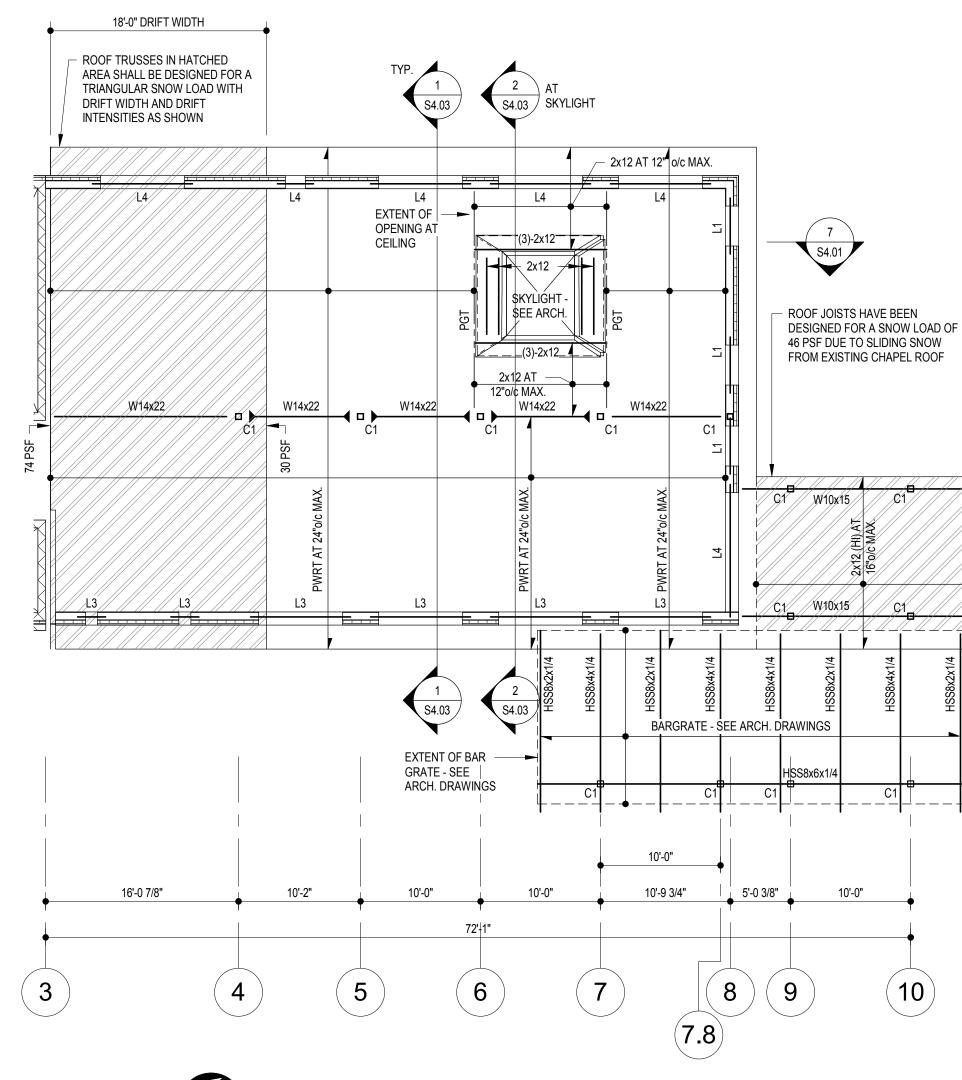
11.REFER TO ARCHITECTURAL AND MECHANICAL DRAWINGS FOR LOCATIONS AND REQUIREMENTS AT LOUVERED OPENINGS BELOW EXTERIOR WINDOWS. FOR FRAMING MODIFICATIONS AT LOUVERED OPENINGS, SEE DETAIL

12.STRUCTURAL STEEL WIDE FLANGE BEAMS SHALL CONFORM TO ASTM A-992, GRADE 50. HOLLOW STRUCTURAL STEEL TUBE SHALL CONFORM TO ASTM A500, GRADE B (Fy=46 KSI). STRUCTURAL STEEL PLATES, ANGLES AND CHANNELS SHALL CONFORM TO ASTM A36.

13.ALL EXPOSED STRUCTURAL STEEL SHALL BE HOT-DIPPED GALVANIZED.

14.STAIRS SHALL BE DESIGNED FOR 100 POUNDS PER SQUARE FOOT LIVE LOAD. SUBMIT SIGNED AND SEALED SHOP DRAWINGS TO ENGINEER FOR REVIEW. STAIRS TO BE METAL PAN. SEE ARCHITECTURAL DRAWINGS FOR RISERS AND TREAD DIMENSIONS. ALL STAIR CHANNELS BEARING ON MASONRY WALLS SHALL REQUIRE 6"x 6"x 1/4" BEARING PLATES AND 1/2"Ø x 8" LONG ANCHOR BOLT WITH 2" HOOK.

15.PROVIDE DOUBLE 8" WIDE x 8" DEEP CONT. BOND BEAM AROUND PERIMETER OF ELEVATOR AND STAIR WITH 2#5 CONT. BOTTOM REINFORCING EACH COURSE AND FILLED SOLID WITH 3000 PSI GROUT AT EACH FLOOR AND ROOF ELEVATION. PROVIDE 90 DEGREE HOOK BARS AT CORNERS.





5

LOW ROOF FRAMING PLAN

ELEVATION TOP OF STRUCTURAL WOOD FRAMED ROOF VARIES. SEE ARCHITECTURAL DRAWINGS FOR CONTROL POINTS. SLOPE TOP CHORD OF WOOD TRUSSES TO ACCOMMODATE ROOF PITCH.

2. STRUCTURAL ROOF DECK OVER WOOD TRUSSES SHALL BE 19/32" EXTERIOR 24/16 EXPOSURE 1 APA RATED PLYWOOD FASTENED TO ROOF TRUSSES. SEE SHEET S1.03 FOR FASTENING SCHEDULE.

3. WALL TYPES ARE SHOWN ON PLAN, THUS:

WOOD STUD WALLS

REINFORCED MASONRY BLOCK WALLS (ASTM C-90)

4. CONTRACTOR SHALL COORDINATE ALL DIMENSIONS AND ELEVATIONS WITH ARCHITECT PRIOR TO CONSTRUCTION. 5. ALL WOOD STUDS SHALL BE NO. 1 / NO. 2 SPRUCE PINE FIR AND SHALL BE COMPRISED OF THE FOLLOWING,

UNLESS NOTED OTHERWISE:

NON-BEARING 2x4 AT 24"o/c (UNO)

WOOD STUD BEARING WALLS SHALL BE BLOCKED AT 4'-0"o/c MAX.

6. WOOD TRUSS LAYOUT AS SHOWN ON PLAN IS DIAGRAMMATIC. ALIGN TRUSSES WITH STUDS BELOW WHERE POSSIBLE. COORDINATE ALL TRUSS LOCATIONS WITH MECHANICAL FLOOR PARAMETERS. PROVIDE DOUBLE JOISTS AND HEADERS AT ALL OPENINGS WIDER THAN TRUSS SPACING. CONNECT HEADER TO DOUBLE TRUSS WITH JOIST HANGERS.

PRE-ENGINEERED WOOD ROOF TRUSSES NOTED, THUS:

PWRT AT WOOD ROOF TRUSSES AT 24"o/c MAX. 24"o/c

PRE-ENGINEERED WOOD GIRDER TRUSS

7. PRE-ENGINEERED TRUSSES SHALL BE DESIGNED FOR THE FOLLOWING MINIMUM SUPERIMPOSED LIVE AND DEAD

LOADS BEYOND THE SELF WEIGHT OF TRUSSES:
 ROOF TYPE
 TOP CHORD
 BOTTOM CHORD

 DEAD
 LIVE
 DEAD
 LIVE
 TYPICAL ROOF 15 PSF 30 PSF 10 PSF 0 PSF

PRE-ENGINEERED WOOD ROOF TRUSS DEFLECTION SHALL NOT EXCEED L/360 OR 1" MAXIMUM FOR LIVE LOAD AND L/240 FOR TOTAL LOAD.

8. FOR WOOD BEAM AND LINTEL SCHEDULES, REFER TO SHEET S1.01. ALL WOOD BEAMS NOTED, THUS: 'B0' ARE TO BE FLUSH FRAMED WITH TRUSS BEARING CONDITIONS. ALL WOOD LINTELS NOTED, THUS: 'LO' ARE TO BE ELEVATED AT

9. STAIRS SHALL BE DESIGNED FOR 100 POUNDS PER SQUARE FOOT LIVE LOAD. SUBMIT SIGNED AND SEALED SHOP DRAWINGS OF STAIR FRAMING AND HANDRAILS PREPARED BY A REGISTERED PROFESSIONAL ENGINEER FROM THE

10.MASONRY WALLS SHALL BE 8" MASONRY (fm = 2500 PSI) REINFORCED PER MASONRY WALL SCHEDULE ON S1.00. 11.REFER TO ARCHITECTURAL AND MECHANICAL DRAWINGS FOR LOCATIONS AND REQUIREMENTS AT LOUVERED

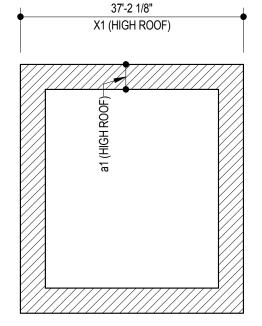
OPENINGS BELOW EXTERIOR WINDOWS. FOR FRAMING MODIFICATIONS AT LOUVERED OPENINGS, SEE DETAIL 12.STRUCTURAL STEEL WIDE FLANGE BEAMS SHALL CONFORM TO ASTM A-992, GRADE 50. HOLLOW STRUCTURAL STEEL TUBE SHALL CONFORM TO ASTM A500, GRADE B (Fy=46 KSI). STRUCTURAL STEEL PLATES, ANGLES AND

CHANNELS SHALL CONFORM TO ASTM A36. 13.ALL EXPOSED STRUCTURAL STEEL SHALL BE HOT-DIPPED GALVANIZED.

14.STAIRS SHALL BE DESIGNED FOR 100 POUNDS PER SQUARE FOOT LIVE LOAD. SUBMIT SIGNED AND SEALED SHOP DRAWINGS TO ENGINEER FOR REVIEW. STAIRS TO BE METAL PAN. SEE ARCHITECTURAL DRAWINGS FOR RISERS AND TREAD DIMENSIONS. ALL STAIR CHANNELS BEARING ON MASONRY WALLS SHALL REQUIRE 6"x 6"x 1/4" BEARING PLATES AND 1/2"Ø x 8" LONG ANCHOR BOLT WITH 2" HOOK.

15.PROVIDE DOUBLE 8" WIDE x 8" DEEP CONT. BOND BEAM AROUND PERIMETER OF ELEVATOR AND STAIR WITH 2#5 CONT. BOTTOM REINFORCING EACH COURSE AND FILLED SOLID WITH 3000 PSI GROUT AT EACH FLOOR AND ROOF ELEVATION. PROVIDE 90 DEGREE HOOK BARS AT CORNERS.

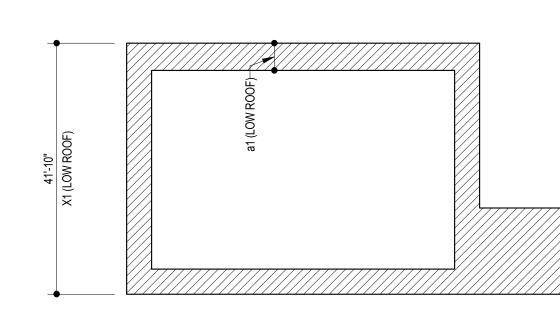
16.FULL CONTINUITY MOMENT CONNECTIONS ARE INDICATED ON PLAN THUS: SHALL HAVE FULL PENETRATION WELDS AT BEAM TO COLUMN TO DEVELOP FULL CAPACITY OF SECTION. SEE DETAIL 6 ON S3.01 FOR ADDITIONAL INFORMATION.





ROOF SHEATHING FASTENING SCHEDULE				
PANEL LOCATION	ZONE 1	ZONE 2		
PANEL END AND EDGE SUPPORTS	6"	3"		
PANEL INTERMEDIATE SUPPORTS	12"	6"		

Wood Fastening Schedule NOT TO SCALE



PART PLAN: LOW ROOF SHEATHING FASTENING

ROOF SHEATHING FASTENING NOTES:

1. SHEATHING FASTENERS SHALL BE 8d RING SHANK NAILS.

2. DIMENSION "a1" SHALL BE 10% OF DIMENSION "X1" OR 40% OF THE

BUILDING HEIGHT (WHICH EVER IS SMALLER), BUT NOT LESS THAN 3 FEET. 3. BLOCK PANEL EDGES PERPENDICULAR TO END WALL FRAMING MEMBERS

WITHIN ZONE 2. BLOCKING SHALL HAVE A MAXIMUM SPACING OF 4'-0" o/c.

4. REFER TO ARCHITECTURAL DRAWINGS FOR BUILDING DIMENSIONS.

STRUCTURAL ENGINEER MORABITO CONSULTANTS 952 RIDGEBROOK ROAD, SUITE 1700 SPARKS, MD 21152 410.467.2377 www.morabitoconsultants.com

PROJECT TEAM

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PROJECT INFORMATION

11

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COMMISSIONING

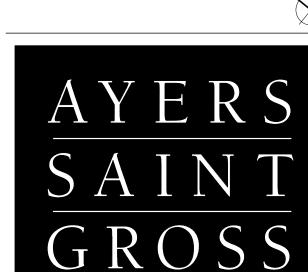
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	REVISIONS			
REV.#	DESCRIPTION	DATE		
·				
		-		
		-		
		-		
	KEY PLAN			

INTERFAITH CENTER



ARCHITECTS + PLANNERS

CONSTRUCTION

DRAWING INFORMATION		
ISSUE DATE:	04/28/17	
SCALE:	As indicated	
JOB NO.:	16072	
DRAWN BY:	CRS	

PROJECT DESIGN PHASE

50% CD SET

DRAWING NAME

DRAWING NUMBER

ROOF FRAMING PLAN

THE GEOTECHNICAL REPORT PREPARED BY HERBST-

EXTENT OF COMPACTED FILL FOR BUILDING STRUCTURE

BENSON & ASSOCIATES DATED JANUARY 6, 2017.

NO PIPES ALLOWED

IN THIS ZONE

• CONTRACTOR TO VERIFY UNDERSLAB PIPE LOCATIONS AND ELEVATIONS

> PIPE NEAR FOOTINGS

[/] 3/4" = 1'-0"

→ NOTE 1 →

DIMENSION SHALL BE 3". REMOVE ALL MORTAR DROPPINGS

AND INSPECT VERT. REINFORCEMENT PRIOR TO PROVIDING

CLOSURE TO RESIST GROUT PRESSURE.

TYPICAL REINFORCED MASONRY CONTRUCTION - VERTICAL REINFORCING ONLY

STABLE AFTER THE BUILDING IS FULLY COMPLETED. IT IS SOLELY THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ERECTION PROCEDURES AND SEQUENCE, AND TO ENSURE THE STABILITY OF THE BUILDING AND ITS COMPONENT PARTS. AND THE ADEQUACY OF TEMPORARY OR INCOMPLETE CONNECTIONS, DURING ERECTING. THIS INCLUDES THE ADDITION OF ANY SHORING, SHEETING, TEMPORARY GUYS, BRACING OR TIEDOWNS THAT MIGHT BE NECESSARY SUCH MATERIAL IS NOT SHOWN ON THE DRAWINGS. IF APPLIED, THEY SHALL BE REMOVED AS CONDITIONS PERMIT AND SHALL REMAIN THE CONTRACTOR'S PROPERTY. THE ENGINEER HAS NO EXPERTISE IN, AND TAKES NO RESPONSIBILITY FOR, CONSTRUCTION MEANS AND METHODS OR JOBSITE SAFETY DURING CONSTRUCTION. PROCESSING AND/OR APPROVED SUBMITTALS MADE BY THE CONTRACTOR WHICH MAY CONTAIN INFORMATION RELATED TO CONSTRUCTION METHODS OF SAFETY ISSUES, OR PARTICIPATION IN MEETINGS WHERE SUCH ISSUES MIGHT BE DISCUSSED, SHALL NOT BE CONSTRUED AS VOLUNTARY ASSUMPTION BY THE ENGINEER OR ANY RESPONSIBILITY OF EACH CONTRACTOR TO FOLLOW ALL APPLICABLE SAFETY CODES AND REGULATIONS DURING ALL PHASES OF CONSTRUCTION. THE ENGINEER IS NOT ENGAGED IN, AND

DOES NOT SUPERVISE CONSTRUCTION.

TESTS DURING EACH EIGHT HOUR SHIFT.

CONTRACTOR RESPONSIBILITIES: THE STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND

GENERAL NOTES

CONTROLLED FILL AND BACKFILL: SAMPLES OF ALL MATERIALS THAT THE CONTRACTOR PROPOSES TO USE FOR COMPACTED FILL SHALL BE APPROVED BY THE GEOTECHNICAL ENGINEER. COMPACTED FILL SHALL CONSIST OF LOCAL MATERIAL FREE OF DELETERIOUS MATTER AND CLASSIFIED CL. SC. GC. GM, OR SM PER ASTM D-2487. THE CONTROL OF THE MOISTURE FOR PLACING THE FILL WILL BE BASED ON THE RESULTS OF COMPACTION TESTS PER ASTM D-1557. ALL COMPACTED FILL SHALL HAVE A DENSITY OF AT LEAST 95% OF THE MODIFIED PROCTOR MAXIMUM DRY DENSITY AS DETERMINED BY ASTM D-1557. PRIOR TO PLACEMENT OF ANY FILLS, THE SITE SHALL BE STRIPPED OF ALL TOPSOIL, VEGETATION, ROCKS, AND ORGANIC MATERIALS AND THE EXPOSED SUBGRADE SHALL BE COMPACTED IN PLACE TO A CONFIRMED DENSITY OF 95% OF THE MODIFIED PROCTOR MAXIMUM DRY DENSITY. FILL MATERIAL SHALL BE PLACED IN LOOSE LIFTS NOT EXCEEDING 8" IN THICKNESS AND SHALL BE MIXED, SPREAD AND PLACED IN SUCH A WAY AS TO PRODUCE A UNIFORM THICKNESS OF MATERIAL AFTER PLACING. EACH LAYER OF FILL SHALL BE COMPACTED WITH A MINIMUM OF 6 COMPLETE PASSES ON ALL PORTIONS OF THE SURFACE OF EACH LIFT OF FILL BY RUBBER-TIRED ROLLERS, SHEEPS-FOOT ROLLERS OR OTHER MECHANICAL EQUIPMENT APPROVED BY THE GEOTECHNICAL ENGINEER. COMPACTED FILL PLACED WITHIN 4 FEET OF STRUCTURES AND PIPES SHOULD BE PLACED IN HORIZONTAL LIFTS NOT TO EXCEED 4 INCHES THICKNESS AND COMPACTED WITH HAND TAMPERS OR LIGHT COMPACTION EQUIPMENT TO THE SAME STANDARD. HEAVY COMPACTION EQUIPMENT SHOULD NOT BE ALLOWED WITHIN 4 FEET OF STRUCTURES UNLESS A MINIMUM 2 FEET DEPTH OF FILL COVERS THE STRUCTURES. WHENEVER IN PLACE DENSITIES ARE FOUND BELOW ACCEPTABLE LIMITS, ADDITIONAL ROLLING TO PRODUCE THE SPECIFIED DENSITIES SHALL BE REQUIRED. THE CONTRACTOR SHALL TAKE ALL MEASURES REQUIRED TO PROVIDE FOR FREE DRAINAGE OF THE SITE AND TO PREVENT PONDING OF WATER. SEDIMENT CONTROL MEASURES SHALL BE MAINTAINED AT ALL TIMES. PLACING OF FILL CONTAINING ORGANIC MATTER; PLACING OF FILL WITH MOISTURE CONTENT TOO HIGH OR TOO LOW FOR PROPER COMPACTION; PLACING OF FILL WHEN FREE WATER IS STANDING ON THE EXISTING FILL SURFACE: PLACING OF FILL IN A FROZEN CONDITION OR ON TOP OF FROZEN MATTER WILL NOT BE PERMITTED. THE SOILS ENGINEER SHALL SUPERVISE THE PLACING OF THE COMPACTED FILL AND ALL THE MATERIAL AND EQUIPMENT USED FOR THIS PURPOSE AND SHALL MAKE SUCH SOILS TESTS AS MAY BE

<u>OUNDATIONS-SPREAD FOOTINGS</u>: BOTTOM OF ALL FOOTINGS SHALL BE A MINIMUM OF 2'-0" BELOW ORIGINAL GRADE OR PLACED IN APPROVED COMPACTED FILL. BOTTOM OF ALL EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 2'-6" BELOW FINISHED GRADE. FOR SOIL BEARING CAPACITY THAT WAS USED IN THE FOUNDATION DESIGN SEE PLAN. SOIL BEARING VALUES MUST BE FIELD VERIFIED BY A REGISTERED GEOTECHNICAL ENGINEER. IF SOIL OF THIS BEARING CAPACITY IS NOT ENCOUNTERED AT THE ELEVATIONS INDICATED ON THE CONTRACT DRAWINGS, FOOTINGS SHALL BE LOWERED OR INCREASED IN SIZE AS DIRECTED BY THE STRUCTURAL ENGINEER. ELEVATIONS SHOWN ON PLAN ARE TO THE BOTTOM OF THE FOOTINGS.

REQUIRED FOR THE COMPLETION OF THE WORK PERFORMING AT LEAST 6 IN PLACE DENSITY

CONCRETE: ALL CONCRETE WORK SHALL CONFORM TO ALL THE PROVISIONS OF THE SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS" (ACI 301-R85) AND TO THE "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" (ACI 318-89). ALL STRUCTURAL CONCRETE SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 3000 PSI UNLESS NOTED OTHERWISE. ADDITIONALLY, THE CONCRETE SHALL CONFORM TO ALL THE PROVISIONS OF "RECOMMENDED PRACTICE FOR HOT WEATHER CONCRETING" (ACI 305-R82) AND "RECOMMENDED PRACTICE FOR COLD WEATHER CONCRETING" (ACI 306-R83). ALL FORMWORK SHALL BE IN ACCORDANCE WITH THE AMERICAN CONCRETE INSTITUTE "FORMWORK FOR CONCRETE" SPECIAL PUBLICATION NO. 4 AND ACI'S "STANDARD RECOMMENDED PRACTICE FOR CONCRETE FORMWORK" (ACI-347-LATEST EDITION). ALL CONCRETE EXPOSED TO THE WEATHEF SHALL HAVE AN AIR ENTRAINMENT OF 5% +/- 1%. THE MAXIMUM WATER CEMENT RATIO W/C SHALL NOT EXCEED 0.53 FOR ALL CONCRETE EXCEPT CONCRETE EXPOSED TO WEATHER WHICH SHALL NOT EXCEED 0.45. NO ADMIXTURES CONTAINING CALCIUM CHLORIDE SHALL BE PERMITTED. THE MAXIMUM SLUMP OF ALL CONCRETE SHALL BE 4". FLOOR SLABS SHALL BE FINISHED TO A MINIMUM FLATNESS F-NUMBER F1 = 30 AND A MINIMUM LEVELNESS F-NUMBER F1 = 25 IN ANY DIRECTION. ALL CONCRETE SHALL BE CURED WITH LIQUID SEALING COMPOUND CONFORMING TO ASTM C-309, TYPE I AND FEDERAL SPECIFICATION TT-C-00800 OR OTHER APPROVED METHOD WHICH IS COMPATIBLE WITH FLOORING ADHESIVES AND OTHER SURFACE TREATMENTS. ALL CONCRETE LEFT EXPOSED AT THE COMPLETION OF THE PROJECT SHALL BI TREATED WITH A CLEAR, PENETRATING ACRYLIC BASE POLYMER CAPABLE OF PREVENTING INFILTRATION OF WATER BORNE CHLORIDES SUCH AS CONSPEC #1 BY CONSPEC MARKETING & MANUFACTURING COMPANY OR APPROVED EQUAL. LOADS GREATER THAN THE DESIGN LIVE LOADS SHALL NOT BE PLACED ON THE STRUCTURE. A CONCRETE STRUCTURE MAY NOT SUPPORT ITS DESIGN LIVE LOAD FOR 28 DAYS. CONTRACTOR SHALL SUPPORT ADJACENT STRUCTURES, UTILITIES, AND EXCAVATIONS AS REQUIRED FOR COMPLETION OF WORK. ONE SET OF COMPRESSIVE TEST CYLINDERS FOR EACH 100 CUBIC YARDS POURED, BUT NOT LESS THAN ONE SET FOR EACH DAY'S POUR AND EACH CLASS OF CONCRETE, ALONG WITH SLUMP TESTS SHALL BE PERFORMED BY A TESTING LABORATORY APPROVED BY THE STRUCTURAL ENGINEER CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR ALL TEMPORARY FORMWORK INCLUDING STRIPPING PROCEDURES FOR CONCRETE FLAT SLABS, SHEETING, SHORING, UNDERPINNING, ETC. SEALED BY A REGISTERED PROFESSIONAL ENGINEER AS PART OF THE CONTRACTOR'S

CONCRETE SLAB ON GRADE CONSTRUCTION: THE CONCRETE SLABS ON GRADE FOR THIS PROJECT HAVE BEEN DESIGNED UTILIZING A MODULUS OF SUBGRADE REACTION "K" EQUAL TO 250 PCI FOR ALL WAREHOUSES, LOADING DOCKS, AND OTHER STORAGE AREAS, AND A MODULUS OF SUBGRADE REACTION "K" EQUAL TO 100 PCI FOR ALL OTHER AREAS OF THE CONCRETE SLABS ON GRADE. PLEASE NOTE THAT THE CONCRETE SLABS ON GRADE THROUGHOUT THIS PROJECT ARE NOT DESIGNED TO SUPPORT THE CRANES USED DURING THE ERECTION OF THE STRUCTURAL STEEL OR CONCRETE TILT-UP WALL BEARING PANELS. IF THE CONTRACTOR ELECTS TO PLACE THE CRANE ON THE CONCRETE SLAB ON GRADE, IT IS THE CONTRACTOR'S RESPONSIBILITY TO TAKE ALL NECESSARY PRECAUTIONS, INCLUDING THE TEMPORARY INSTALLATION OF WOOD CRIBBING ON THE SLAB, IN ORDER TO PREVENT CRACKS FROM FORMING IN THE SLABS ON GRADE. ALL CRACKS WHICH FORM IN THE CONCRETE SLABS ON GRADE DUE TO THE CRANE BEING PLACED ON THE SLAB WILL BE REPLACED OR REPAIRED TO THE APPROVAL OF THE STRUCTURAL ENGINEER AND OWNER AT THE CONTRACTOR'S EXPENSE

INFORCING STEEL: REINFORCING STEEL SHALL BE DEFORMED BARS IN ACCORDANCE WITH ASTM A-615, GRADE 60. BENDS ARE TO BE FABRICATED AS PER DETAILS. PLACE MAIN REINFORCING STEEL SO AS TO PROVIDE 3" MINIMUM COVER FOR FOUNDATIONS POURED ON EARTH, 2" MINIMUM COVER FOR BEAMS AND COLUMNS, 3/4" MINIMUM COVER FOR SLABS AND 1 1/2" FOR ALL REBAR IN EXPOSED CONCRETE (EXCEPT AS OTHERWISE DETAILED). ALL BEAM AND SLAB STEEL SHALL HAVE A MINIMUM EXTENSION INTO THE SUPPORTS IN ACCORDANCE WITH THE LATEST ADDITION OF THE ACI CODE, PROVIDE ACCESSORIES AND BAR SUPPORTS IN ACCORDANCE WITH THE MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES (ACI 315-80). TOP REINFORCING IN PARKING DECKS AND ALL OTHER REINFORCING SHOWN ON THE CONTRACT DOCUMENTS SHALL BE EPOXY-COATED CONFORMING TO ASTM A-775 FOR BARS AND ASTM A-884 FOR WELDED WIRE FABRIC. WHERE REQUIRED, DAMAGED AND CUT EPOXY-COATING SHALL BE REPAIRED WITH PATCHING MATERIAL CONFORMING TO ASTM A-775. EPOXY-COATED REINFORCING BARS SHALL REST ON COATED WIRE BAR SUPPORTS MADE OF DILLECTIC MATERIAL FOR A MINIMUM DISTANCE OF 2 INCHES FROM POINT OF CONTACT WITH EPOXY-COATED REBARS. EPOXY-COATED REINFORCING BARS SHALL BE FASTENED WITH NYLON-, EPOXY-, OR PLASTIC-COATED TIE WIRE. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-185, GRADE 60. UNLESS OTHERWISE NOTED, WWF REINFORCING SHALL BE PLACED AT MID-DEPTH OF SLABS ON GRADE AND DRAPED OVER SUPPORTS IN CONCRETE SLABS ON CENTERING. END LAPS OF ALL WWF REINFORCING SHALL BE LAPPED 8" MINIMUM. CONCRETE ENGINEERED REINFORCING FIBERS SHALL BE POLYPROPYLENE COLLATED, FIBRILLATED FIBERS FROM FIBERMESH, INC. INSTALLED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.

STRUCTURAL STEEL: STRUCTURAL STEEL PLATES, ANGLES, CHANNELS, BARS, AND ROLLED S, M AND HP SHAPES SHALL CONFORM TO ASTM A-36. ROLLED WIDE FLANGE SHAPES SHALL CONFORM TO ASTM A-992, GRADE 50; ASTM A-572, GRADE 50; ASTM AH-36, GRADE 50; OR ASTM A-588, GRADE 50. STRUCTURAL STEEL TUBULAR SHAPES SHALL CONFORM TO ASTM A-500, GRADE B (FY=46KSI) AND STRUCTURAL STEEL PIPES SHAPES SHALL CONFORM TO ASTM A-53, TYPES E OR S, GRADE B (FY=35KSI). ANCHOR BOLTS SHALL CONFORM TO ASTM F1554, GRADE 36. ALL CONNECTIONS WITH SLOTTED AND OVERSIZE HOLES SHALL HAVE SLIP CRITICAL CONNECTIONS ALL OTHER CONNECTIONS MAY BE BEARING TYPE CONNECTIONS. ALL BOLTS SHALL CONFORM TO ASTM A-325. WELDS SHALL CONFORM TO ALL THE PROVISIONS OF THE STRUCTURAL WELDING CODE. AWS D1.1-2002 OF THE AMERICAN WELDING SOCIETY EXCEPT SECTIONS 2.3, 2.4, 2.5. 8.13.1.2 AND 9. HEADED STUD TYPE SHEAR CONNECTORS SHALL BE COLD FINISHED CARBON STEEL COMPLYING WITH ASTM A-108. GRADE 1015 OR 1020. WITH DIMENSIONS COMPLYING WITH AISC SPECIFICATIONS. NO OPENINGS IN BEAMS OTHER THAN SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE PERMITTED WITHOUT WRITTEN APPROVAL FROM THE STRUCTURAL ENGINEER. PROVIDE 2 MILS OF ONE OF THE FOLLOWING SHOP PAINTS: NO. 769 GREY PRIMEF MANUFACTURED BY RUST-OLEUM CORPORATION; NO. 769 GREY PRIMER MANUFACTURED BY TNEMEC COMPANY AND WETSALL PRIMER MANUFACTURED BY FARBOIL COMPANY. PROVIDE SHOP AND FIELD INSPECTION OF ALL STRUCTURAL STEEL BY A TESTING LABORATORY APPROVED BY THE STRUCTURAL ENGINEER.

ONRY: SOLID MASONRY SHALL BE GRADE N1 IN ACCORDANCE WITH ASTM C-90 AND MAY BE 75% SOLID UNLESS OTHERWISE NOTED. HOLLOW MASONRY UNITS SHALL BE GRADE N1 CONFORMING TO ASTM C-90. MASONRY UNITS SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 1900 PSI BASED ON THE NET CROSS SECTIONAL AREA OF THE INDIVIDUAL BLOCK UNITS. ALL MORTAR SHALL BE TYPE "S" CONFORMING TO ASTM C-270 WITH MINIMUM COMPRESSIVE STRENGTH OF 1800 PSI AT 28 DAYS. PROVIDE A MINIMUM OF 3 COURSES OF SOLID BRICK OR ONE COURSE OF 100% SOLID BLOCK UNDER WALL BEARING ENDS OF ALL JOISTS AND SLABS, THE FULL WIDTH OF THE WALL, UNLESS NOTED. IN BEARING WALLS, PROVIDE SOLID BRICK OR 100% BLOCK EXTENDING 8" BEYOND WALL OPENINGS THE FULL WALL THICKNESS DOWN TO THE FLOOR, UNLESS NOTED. ALL PORTIONS OF MASONRY WALLS HAVING A HORIZONTAL CROSS SECTION OF 4 SQ. FT. OR LESS SHALL BE OF SOLID MASONRY DOWN TO FOOTINGS. PROVIDE HORIZONTAL MASONRY REINFORCING (DUR-O-WAL OR EQUAL) AT 16" o/c. IN ALL MASONRY WALLS UNLESS NOTED. ALL MASONRY WALLS SHALL HAVE CONTROL JOINTS AT 40'-0" o/c. MAXIMUM. SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS. ALL MORTAR JOINTS IN MASONRY WALLS (HORIZONTAL AND VERTICAL) SHALL BE FILLED 100% WITH MORTAR. USE BUCKETS TO MEASURE MATERIALS FOR MIXING MORTAR.

LINTELS: ALL OPENINGS IN NEW WALLS AND PARTITIONS ARE TO BE PROVIDED WITH LINTELS. LINTELS SHALL BE STONE, CONCRETE, SLAG CONCRETE, OR STRUCTURAL STEEL. PROVIDE 4" MINIMUM END BEARING FOR LINTELS IN NON-BEARING PARTITIONS AND 8" MINIMUM END BEARING FOR LINTELS IN ALL EXTERIOR WALLS AND BEARING PARTITIONS. FOR ANY OPENING NOT SPECIFICALLY SHOWN. PROVIDE ONE 4" x 3 1/2" x 5/16" (LLV) ANGLE FOR EACH 4" OF WALL THICKNESS FOR SPANS NOT EXCEEDING 6'-0"; ONE 6" x 3 1/2" x 5/16" (LLV) ANGLE FOR EACH 4" OF WALL THICKNESS FOR SPANS EXCEEDING 6'-0" BUT LESS THAN 8'-0" OR PRECAST CONCRETE LINTELS AS DIRECTED BY THE ARCHITECT. PRECAST CONCRETE LINTELS SHALL HAVE ONE #4 TOP AND BOTTOM FOR EACH 4" OF WALL THICKNESS FOR SPANS NOT EXCEEDING 6'-0": ONE #5 TOP AND BOTTOM FOR EACH 4" OF WALL THICKNESS FOR SPANS EXCEEDING 6'-0" BUT LESS THAN 8'-0". ALL PRECAST CONCRETE LINTELS SHALL ALSO BE REINFORCED WITH #2 WIRE TIES AT 8"o/c. SEE ARCHITECTURAL, MECHANICAL, ELECTRICAL AND STRUCTURAL DRAWINGS FOR LOCATIONS OF LINTELS. CONSULT STRUCTURAL ENGINEER FOR LINTEL REQUIREMENTS FOR ALL NEW OPENINGS IN EXISTING WALLS.

POST-INSTALLED ANCHORS: POST-INSTALLED ANCHORS SHALL ONLY BE USED WHERE SPECIFIED ON HE CONSTRUCTION DOCUMENTS. SUBSTITUTION REQUESTS FOR ALTERNATE PRODUCTS MUST BE APPROVED IN WRITING BY THE STRUCTURAL ENGINEER OF RECORD PRIOR TO USE. CONTRACTOR SHALL PROVIDE CALCULATIONS DEMONSTRATING THAT THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING THE PERFORMANCE VALUES OF THE SPECIFIED PRODUCT. SUBSTITUTIONS WILL BE EVALUATED BY THEIR HAVING AN ICC ESR SHOWING COMPLIANCE WITH THE RELEVANT BUILDING CODE FOR SEISMIC USES, LOAD RESISTANCE, INSTALLATION CATEGORY, AND AVAILABILITY OF COMPREHENSIVE INSTALLATION INSTRUCTIONS. ADHESIVE ANCHOR EVALUATION WILL ALSO CONSIDER CREEP, FATIGUE, IN-SERVICE TEMPERATURE AND INSTALLATION TEMPERATURE.

11

ANCHOR CAPACITY IS HIGHLY DEPENDANT UPON SPACING BETWEEN ADJACENT ANCHORS AND PROXIMITY OF ANCHORS TO EDGE OF CONCRETE/MASONRY. INSTALL ANCHORS IN ACCORDANCE WITH SPACING AND EDGE CLEARANCES INDICATED ON THE DRAWINGS. IF EDGE DISTANCES OR ANCHOR SPACING IS NOT SPECIFIED ON THE DRAWINGS, PROVIDE THE FOLLOWING MINIMUM DISTANCES.

A) EDGE DISTANCES a. ADHESIVE ANCHORS: 2 TIMES THE ANCHOR EMBEDMENT LENGTH b. UNDERCUT ANCHORS: 2.5 TIMES THE ANCHOR EMBEDMENT LENGTH c. EXPANSION ANCHORS (SLEEVE OR WEDGE): 4 TIMES THE ANCHOR EMBEDMENT

B) ANCHOR SPACINGS a. ALL ANCHORS: 3 TIMES THE ANCHOR EMBEDMENT

ANCHORS SHALL BE INSTALLED BY QUALIFIED PERSONNEL IN ACCORDANCE WITH THE CONSTRUCTION DOCUMENTS, BUILDING CODE, AND MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS (MPII). THE STRUCTURAL ENGINEER OF RECORD MUST RECEIVE DOCUMENTED CONFIRMATION THAT ALL OF THE CONTRACTOR'S PERSONNEL WHO INSTALL POST-INSTALLED ANCHORS HAVE BEEN PROPERLY TRAINED PRIOR TO THE COMMENCEMENT OF INSTALLING FOR EACH SPECIFIC PRODUCT. INSTALLATION OF ADHESIVE ANCHORS HORIZONTALLY OR UPWARDLY INCLINED TO SUPPORT <u>SUSTAINED TENSION LOADS</u> (AS DETERMINED BY THE ENGINEER) SHALL BE PERFORMED BY PERSONNEL CERTIFIED BY THE ACI-CRSI "ADHESIVE ANCHOR INSTALLER

CERTIFICATION PROGRAM". A) ADHESIVE ANCHORS INSTALLED IN HORIZONTAL OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS SHALL BE CONTINUOUSLY INSPECTED DURING INSTALLATION BY AN INSPECTOR SPECIALLY APPROVED FOR THAT PURPOSE BY THE BUILDING OFFCIAL. THE SPECIAL INSPECTOR SHALL FURNISH A REPORT TO THE STRUCTURAL ENGINEER OF RECORD AND BUILDING OFFICIAL THAT THE WORK COVERED BY THE REPORT HAS BEEN PROPERLY PERFORMED AND THAT THE MATERIALS USED AND THE INSTALLATION PROCEDURES USED CONFORM WITH THE APPROVED CONSTRUCTION DOCUMENTS AND THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS (MPII).

ADHESIVE ANCHORS SHALL BE INSTALLED IN CONCRETE HAVING A MINIMUM AGE OF 21 DAYS AT TIME OF ANCHOR INSTALLATION. IF HIGH-EARLY STRENGTH CONCRETE MIXES ARE SPECIFIED, CONTACT THE STRUCTURAL ENGINEER OF RECORD FOR APPROVAL OF MINIMUM INSTALLATION AGE. EXISTING REINFORCING BARS OR PRESTRESSING STEEL IN THE CONCRETE STRUCTURE MAY CONFLICT WITH SPECIFIC ANCHOR LOCATIONS. UNLESS NOTED ON THE DRAWINGS THAT THE BARS CAN BE CUT, THE CONTRACTOR SHALL REVIEW THE EXISTING STRUCTURAL DRAWINGS AND SHALL UNDERTAKE TESTING TO LOCATE THE POSITION AND DEPTH OF THE REINFORCING BARS OR PRESTRESSING AT THE LOCATIONS OF THE CONCRETE ANCHORS, BY HILTI FERROSCAN, GPR, X-RAY CHIPPING OR OTHER MEANS.

EXCEPT WHERE INDICATED ON THE DRAWINGS, POST-INSTALLED ANCHORS SHALL CONSIST OF THE FOLLOWING ANCHOR TYPES. ANCHORS EXPOSED TO WEATHER AND AT SILL PLATES SHALL BE STAINLESS STEEL.

a) MECHANICAL ANCHORS IN CRACKED OR UNCRACKED CONCRETE USE: (1) HILTI KWIK BOLT-TZ EXPANSION ANCHORS (2) HILTI KWIK HUS-EZ AND KWIK HUS EZ-I SCREW ANCHORS b) ADHESIVE ANCHORS IN CRACKED AND UNCRACKED CONCRETE USE (1) HILTI HIT-HY 200 SAFE SET SYSTEM WITH HILTI HIT-Z ROD

(2) HILTI HIT-HY 200 SAFE SET SYSTEM WITH HILTI HOLLOW DRILL BIT SYSTEM WITH HAS-E THREADED ROD c) REBAR DOWELING INTO CONCRETE: (1) HILTI HIT-HY 200 SAFE SET SYSTEM WITH HILTI HOLLOW DRILL BIT SYSTEM

d) ADHESIVE ANCHORS IN HOLLOW / GROUTED / MULTI-WYTHE MASONRY USE: (1) HILTI HIT-HY 70 MASONRY ADHESIVE ANCHORING SYSTEM e) MECHANICAL ANCHORS IN GROUTED MASONRY USE: (1) HILTI KWIK HUS-EZ SCREW ANCHORS (2) HILTI KWIK BOLT-3 EXPANSION ANCHORS

Ft = 425 PSI Fc = 1150 PSI

GALVANIZED PER ASTM A-653M, DESIGNATION G185 COATING.

STRUCTURAL LUMBER: STRUCTURAL WOOD STUDS AND JOISTS SHALL BE NO.1/NO. 2 SPRUCE-PINE-FIR WITH A MAXIMUM WATER CONTENT OF 19% AND THE FOLLOWING MINIMUM Fb = 875 PSI Fc = 425 PSI

Fv = 135 PSI E = 1,400,000 PSI STRUCTURAL FLOOR SHEATHING SHALL BE 23/32" TONGUE AND GROOVE EXPOSURE 1 APA GROUP 1 RATED STURD-I-FLOOR GLUED AND NAILED TO WOOD TRUSSES AS SPECIFIED ON PLAN. STRUCTURAL ROOF SHEATHING SHALL BE 19/32" EXTERIOR 24/16 EXPOSURE 1 APA RATED PLYWOOD FASTENED TO ROOF TRUSSES, WITH PLYWOOD SHEATHING CLIPS BETWEEN SUPPORTIING MEMBERS WHERE EDGES OF THE PLYWOOD ABUT ONE ANOTHER. WHERE WOOD JOISTS FRAME INTO BEAMS, USE 16 GAUGE STANDARD JOIST HANGERS AND 10d NAILS, UNO. PROVIDE SOLID WOOD BLOCKING OR 16 GAUGE DIAGONAL X-BRIDGING BETWEEN ALL FLOOR JOISTS OR TRUSSES AT 8'-0" o/c AND BETWEEN ALL ROOF JOISTS OR TRUSSES AT 10'-0" o/c MAXIMUM. ALL ROOF TRUSSES WITH OVERHANGS, ALL ROOF RAFTERS IN CATHEDRAL CEILING AREAS, ALL FLOOR JOISTS/TRUSSES IN EXTERIOR BAI CONIES. EVERYWHERE FIRST FLOORS AND DECKS ARE FLEVATED ABOVE THE PERIMETER GRADE ELEVATIONS AND ALL OTHER HORIZONTAL SURFACES EXPOSED TO WIND UPLIFT SHALL BE SECURED TO THE BUILDING FRAMING WITH 18 GAUGE HURRICANE ANCHORS AND 10d NAILS. ALL LUMBER. BLOCKING. FURRING AND OTHER WOOD IN CONTACT WITH CONCRETE. MASONRY, THE GROUND OR EXPOSED TO THE WEATHER SHALL BE PRESSURE TREATED WITH WATER-BORNE PRESERVATIVES IN ACCORDANCE WITH THE AMERICAN WOOD PRESERVERS' INSTITUTE STANDARD AWPA-P5. LUMBER SHALL BE KILN-DRIED TO 15% MOISTURE CONTENT AFTER TREATMENT. COAT ALL CUT SURFACES OF TREATED LUMBER WITH AN APPROVED PRESERVATIVE. ALL CONNECTIONS OF BEAMS TO BEAMS, BEAMS TO COLUMNS, BRACING TO STRUCTURE AND COLUMNS TO FOUNDATIONS SHALL BE ACCOMPLISHED USING STEEL CONNECTIONS OF TYPES SHOWN ON THESE DRAWINGS. ALL PLATES AND BARS USED IN THESE CONNECTIONS SHALL BE FABRICATED FROM STRUCTURAL STEEL CONFORMING TO ASTM A-36 WITH BOLTS CONFORMING TO ASTM A-307. ALL CONNECTIONS INCLUDING PLATES, DOWELS, BOLTS AND NAILS EXPOSED TO THE WEATHER SHALL BE GALVANIZED. ALL CONNECTIONS TO PRESSURE TREATED WOOD SHALL BE

WOOD TRUSSES: STRUCTURAL ROOF TRUSSES SHALL BE DESIGNED PER THE STANDARD DESIGN SPECIFICATION FOR METAL PLATE CONNECTED WOOD TRUSSES TPI-02 AS PREPARED PER THE TRUSS PLATE INSTITUTE, INC. THE WOOD TRUSSES SHALL BE CUSTOM DESIGNED TO FIT THE DIMENSIONS AND LOADS INDICATED ON THE PLANS. ALL DESIGNS SHALL BE IN ACCORDANCE WITH THE ALLOWABLE LOAD VALUES SHOWN ON THE PLANS. ALL PREFABRICATED TRUSSES SHALL HAVE ERECTION BRACING, STRUT BRACING, AND BRIDGING AS REQUIRED BY THE MANUFACTURER TO RESIST ALL CONSTRUCTION AND BUILDING LOADS. SHOP DRAWINGS INCLUDING DESIGN CALCULATIONS, MEMBER FORCES AND STRESS CONTROL POINTS SHALL BE SEALED BY A REGISTERED PROFESSIONAL ENGINEER AND SUBMITTED TO THE STRUCTURAL ENGINEER FOR

HOP DRAWINGS: SHOP DRAWINGS FOR ALL STRUCTURAL ELEMENTS SHOWN ON THE CONTRACT DOCUMENTS MUST BE SUBMITTED BY GENERAL CONTRACTOR AND REVIEWED BY THE ENGINEER. ALL CONTRACTOR MODIFICATIONS (INCLUDING PRODUCTS SUBMISSION) MUST BE IDENTIFIED IN WRITING AS A PROPOSED "AS EQUAL" CHANGES AT TIME OF SUBMISSION. IF A CONTRACTOR OR OWNER FAILS TO SUBMIT THE SHOP DRAWINGS OR FAILS TO FOLLOW THE ABOVE "AS EQUAL" PROCEDURE. THE FIRM MORABITO CONSULTANTS, INC. WILL NOT BE RESPONSIBLE FOR THE STRUCTURAL CERTIFICATION AND DESIGN OF THE PROJECT. SHOP DRAWINGS ARE REVIEWED BY THE ENGINEER AS A CONVENIENCE TO THE CONTRACTOR AND ARE NOT A CONTRACT DOCUMENT. INSPECTION: ALL WORK SPECIFIED HEREIN SHALL BE INSPECTED IN ACCORDANCE WITH THE

BUILDING CODE AND ALL LOCAL ORDINANCES. THE OWNER OR CONTRACTOR SHALL HIRE AN EXPERIENCED QUALIFIED INSPECTOR TO PERFORM ALL REQUIRED INSPECTION WORK. INSPECTION SHALL CONSIST OF VISUAL OBSERVATIONS OF MATERIALS, EQUIPMENT OR CONSTRUCTION WORK FOR THE PURPOSE OF ASCERTAINING THAT THE WORK IS IN SUBSTANTIAL CONFORMANCE WITH THE CONTRACT DOCUMENTS AND WITH THE DESIGN INTENT. THE ENGINEER WILL NOT PERFORM THE REQUIRED INSPECTION AS PART OF THIS PRESENT CONTRACT WITH THE ARCHITECT/OWNER. UNDER THIS PRESENT CONTRACT, THE ENGINEER MAY VISIT THE SITE TO ASCERTAIN GENERAL CONFORMANCE TO THE CONTRACT DOCUMENTS. HOWEVER, SUCH VISITS SHALL NOT BE RELIED UPON BY OTHERS AS ACCEPTANCE OF THE WORK, NOR SHOULD IT BE CONSTRUED TO RELIEVE THE CONTRACTOR IN ANY WAY FROM HIS OBLIGATIONS AND RESPONSIBILITIES UNDER THE CONSTRUCTION CONTRACT. HOWEVER, IF DESIRED, MORABITO CONSULTANTS, INC. MAY BE HIRED UNDER A SEPARATE CONTRACT TO PERFORM THIS INSPECTION WORK.

ESIGN WITHOUT CONSTRUCTION REVIEW: IT IS AGREED THAT IF MORABITO CONSULTANTS, INC.' S PROFESSIONAL SERVICES DO NOT EXTEND TO OR INCLUDE THE REVIEW OR SITE OBSERVATION OF THE CONTRACTOR'S WORK OR PERFORMANCE, THEN THE OWNER WILL DEFEND, INDEMNIFY AND HOLD HARMLESS MORABITO CONSULTANTS, INC., FROM ANY CLAIM OR SUIT WHATSOEVER, INCLUDING BUT NOT LIMITED TO ALL PAYMENTS, EXPENSES OR COSTS INVOLVED, ARISING FROM OR ALLEGED TO HAVE ARISEN FROM THE CONTRACTOR'S PERFORMANCE OR THE FAILURE OF THE CONTRACTOR'S WORK TO CONFORM TO THE DESIGN INTENT AND THE CONTRACT DOCUMENTS MORABITO CONSULTANTS, INC., AGREES TO BE RESPONSIBLE FOR ITS OWN OR ITS EMPLOYEES' NEGLIGENT ACTS, ERRORS OR OMISSIONS.

VNERSHIP OF DOCUMENTS: THE CONTRACTOR ACKNOWLEDGES THESE PLANS AND SPECIFICATIONS PREPARED BY MORABITO CONSULTANTS, INC., AS INSTRUMENTS OF PROFESSIONAL SERVICE. NEVERTHELESS, THE PLANS AND SPECIFICATIONS PREPARED UNDER THIS AGREEMENT SHALL REMAIN THE PROPERTY OF MORABITO CONSULTANTS, INC. UPON COMPLETION OF THE WORK. THE CONTRACTOR AGREES TO HOLD HARMLESS AND INDEMNIFY MORABITO CONSULTANTS, INC., AGAINST ALL DAMAGES, CLAIMS, AND LOSSES, INCLUDING DEFENSE COSTS, ARISING OUT OF ANY REUSE OF THE PLANS AND SPECIFICATIONS WITHOUT THE WRITTEN AUTHORIZATION OF MORABITO CONSULTANTS, INC.

FLAT-ROOF SNOW LOAD

c = 3000 PSI (FOOTINGS UNO) fc = 4500 PSI (RET. WALL FOOTINGS) 2. SNOW EXPOSURE FACTOR 3. SNOW LOAD IMPORTANTANCE FACTOR I = 1.1 f'c = 3500 PSI (SLAB ON GRADE) Fy = 50000 PSI

COMMON ROOMS = 100 PSF OFFICES = 50 PSF CORRIDORS = 100 PSF ELEVATOR LOBBY = 100 PSF STAIRS = 100 PSF ROOF = 30 PSF + SNOW DRIFT

PARTITIONS = 20 PSF

<u>EARTHQUAKE LOAD IBC-2015</u> SEISMIC USE GROUP = III, IE = 1.00 2. Ss \leq 0.136g, S1 \leq 0.052g 3. SDS = 0.109, SD1 = 0.059 4. SITE CLASS = C

5. SEISMIC DESIGN CATEGORY A 6. DUAL SYSTEM WITH INTERMEDIATE MOMENT FRAMES CAPABLE OF RESISTING AT LEAST 25% OF PRESCRIBED SEISMIC FORCES IN NORTH-SOUTH DIRECTION AND ORDINARY REINFORCED MASONRY SHEAR WALLS IN EAST-WEST DIRECTION 7. DESIGN BASE SHEAR = 11 K 8. Cs = 0.055

Pg = 20 PSF

Pf = 22 PSF

Ce = 1.0

9. RESPONSE MODIFICATION FACTOR = 3.0 IN N-S = 2.0 IN E-W 10. EQUIVALENT LATERAL FORCE DESIGN METHOD

1. BASIC WIND SPEED = 115 MPH 2. BUILDING CATEGORY = II 3. IMPORTANCE FACTOR = 1.00 WIND EXPOSURE = B 5. INTERNAL PRESSURE COEFFICIENT = ±0.18 COMPONENTS AND CLADDING = ±30 PSF (EXTERIOR WALLS) ±35 PSF (WALL CORNERS)

PROJECT INFORMATION

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REVISIONS DESCRIPTION **KEY PLAN**

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CONSTRUCTION

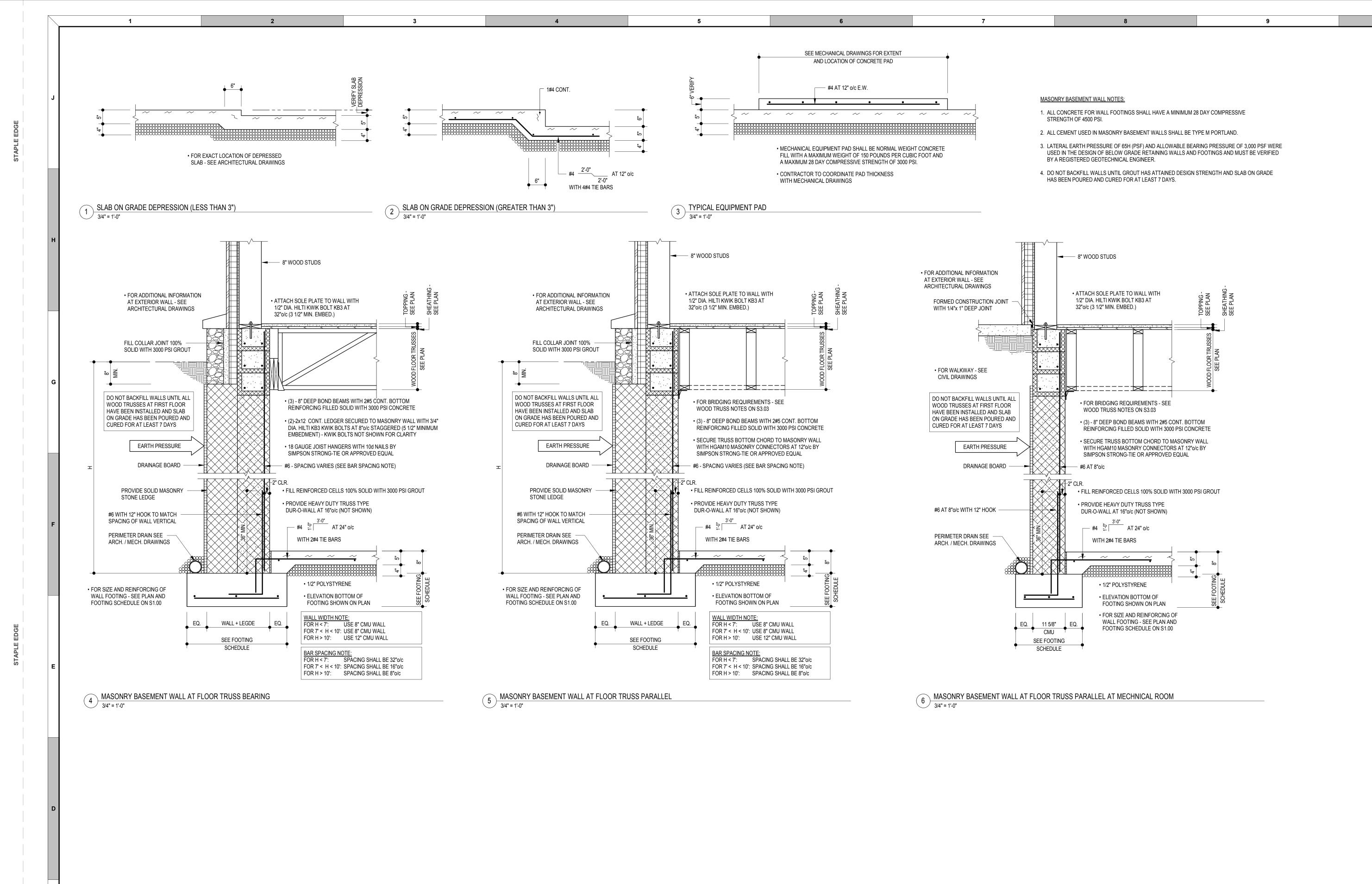
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DRAWING NAME FOUNDATION DETAILS AND

DRAWING NUMBER

GENERAL NOTES



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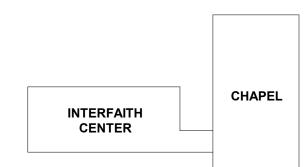
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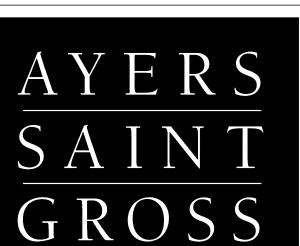
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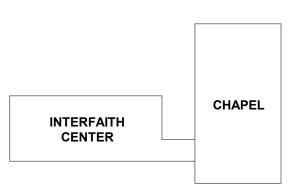
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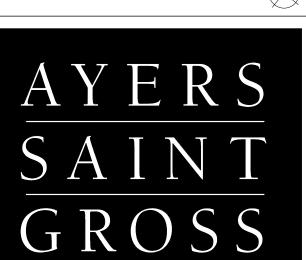
REVISIONS

REV. # DESCRIPTION DATE

Revision 1 Date 1

KEY PLAN





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 DRAWING INFORMATION

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 04/28/17

 SCALE:
 3/4" = 1'-0"

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 16072

 DRAWN BY:
 CRS

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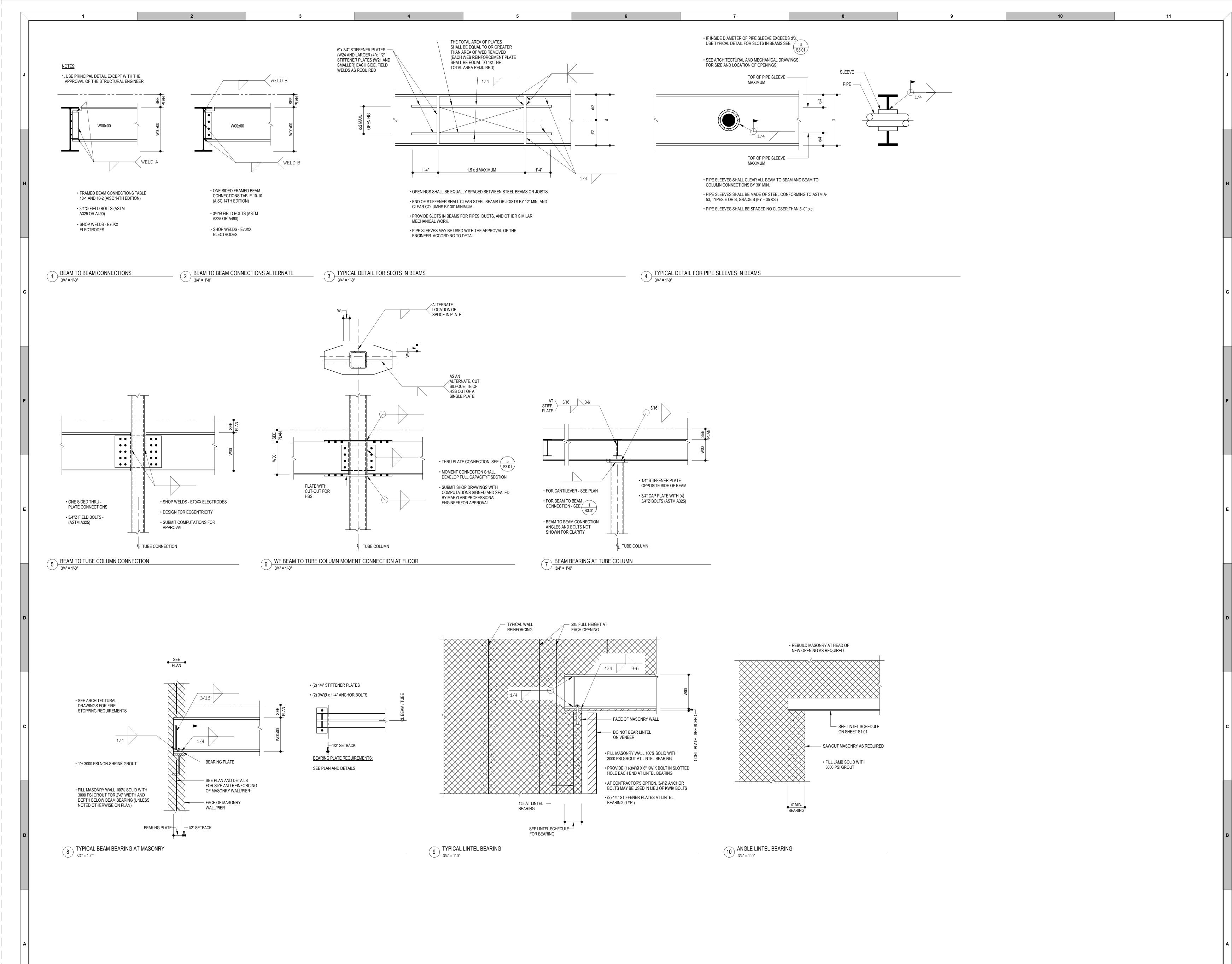
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50% CD SET

DRAWING NAME

FOUNDATION DETAILS

S2.03



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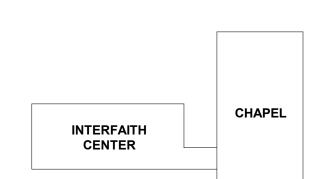
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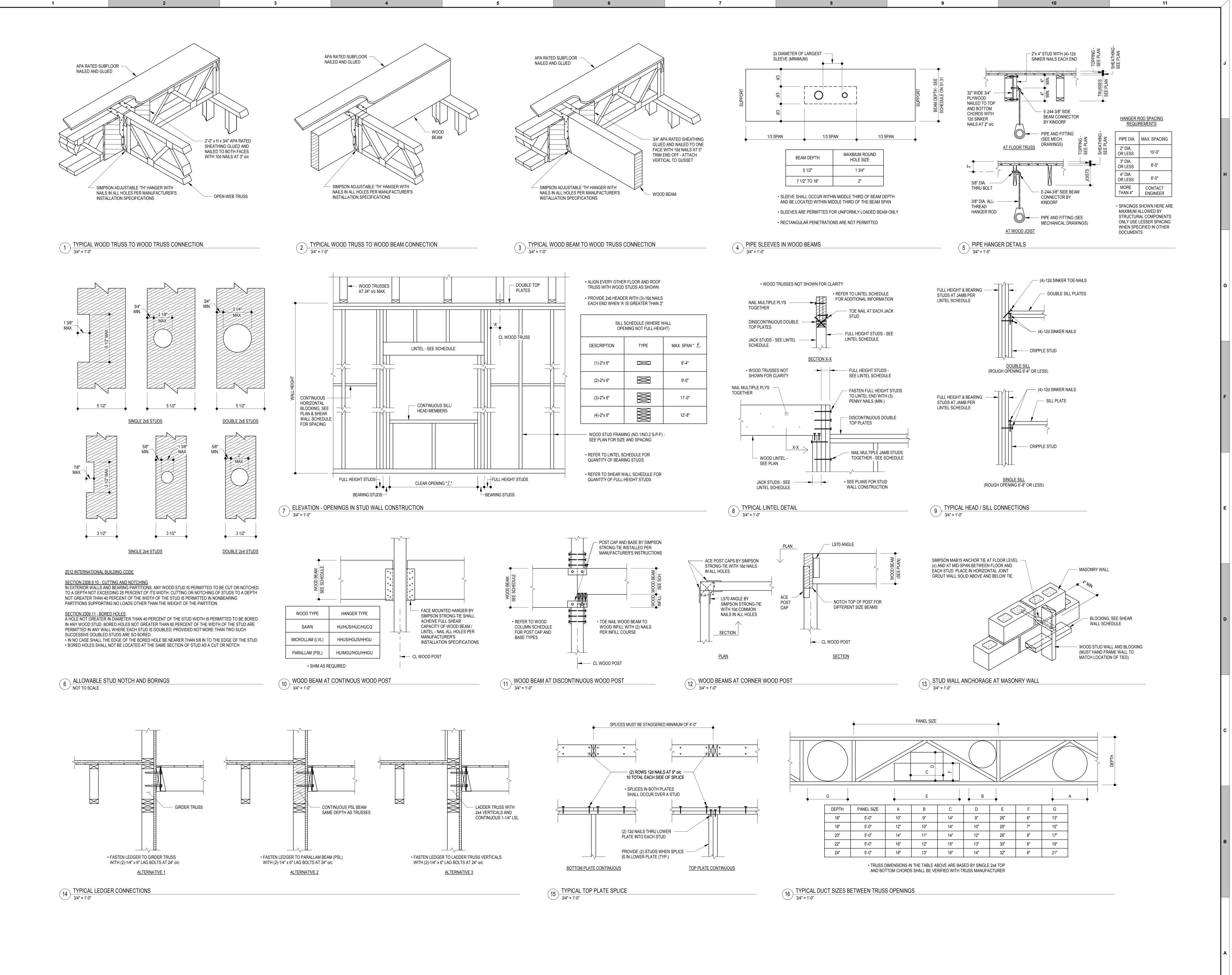
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TYPICAL STEEL DETAILS

DRAWING NAME

DRAWING NUMBER **S3.01**



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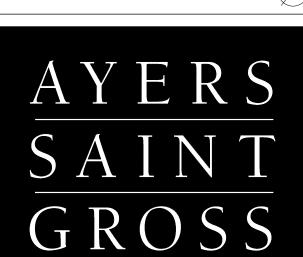
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REV. # DESCRIPTION DATE

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JOB NO.:	16072
DRAWN BY:	CRS
PROJECT DES	SIGN PHASE

PROJECT DESIGN FITASE

50% CD SET

TYPICAL WOOD FRAMING

DRAWING NAME

DETAILS

DRAWING NUMBER

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S3.02

-BEARING STUDS

• FOR ADDITIONAL INFORMATION AT FRAMING AROUND WALL OPENINGS - SEE $\frac{1}{30.02}$

- LAP ALL TOP PLATES AT WALL CORNERS AND

INTERSECTIONS WITH

(4) 12d NAILS MINIMUM

FRAMING PLANS

AND SCHEDULE

- CRIPPLE STUD -- SHEAR PANEL 2x6 HEADER CONTRACTOR ALTERNATIVE 1

- CRIPPLE STUD -2x6 HEADER

CONTRACTOR ALTERNATIVE 2

• FASTEN SHEAR TRUSS TO TOP AND BOTTOM PLATES WITH 12d NAILS STAGGERED AT 4" o/c.

WEB STIFFENER

- 2x6 EVERY

OTHER TRUSS

CRIPPLE STUD -

• PROVIDE CRIPPLE STUDS AT INTERMEDIATE STUDS AND 2x6 HEADER AS REQUIRED BY

• PRE-ENGINEERED SHEAR TRUSS ELEMENT BY TRUSS MANUFACTURER TO

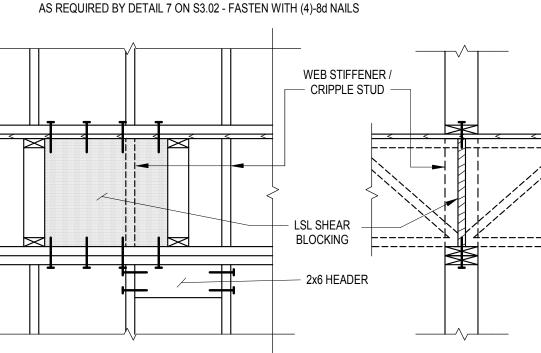
BE DESIGNED TO WITHSTAND A HORIZONTAL SHEAR FORCE OF 250#/FT

DETAIL 7 ON S3.02 - FASTEN WITH (4)-8d NAILS.

• FIELD FABRICATED 2x4 SHEAR PANELS AS SHOWN WITH 15/32 OSB WITH 8d NAILS AT 4" o/c - VERTICAL TO ALIGN WITH INTERMEDIATE STUDS

• FASTEN PANELS TO TOP AND BOTTOM PLATES WITH 12d NAILS AT 4" o/c. • PROVIDE CRIPPLE STUDS AT INTERMEDIATE STUDS AND 2x6 HEADER

5



CONTRACTOR ALTERNATIVE 3

• FASTEN SHEAR BLOCKING TO TOP AND BOTTOM PLATES WITH #12 x 4" LONG WOOD SCREWS AT 4" o/c

• 1-1/4" LAMINATED STRAND LUMBER (LSL) SHEAR BLOCKING TO MATCH DEPTH OF TRUSS

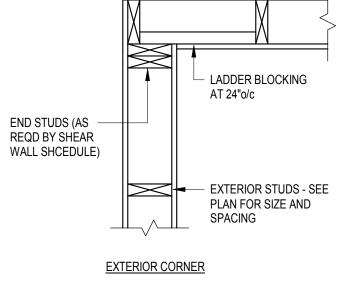
• PROVIDE WEB STIFFENERS AT INTERMEDIATE STUDS AND 2x6 HEADER

AS REQUIRED BY DETAIL 7 ON S3.02 - FASTEN WITH (4)-8d NAILS

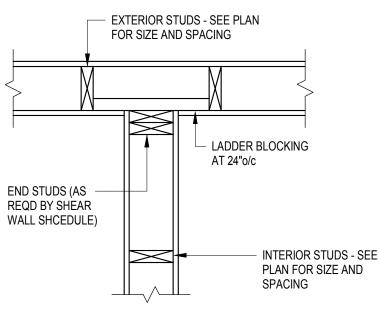
CONTRACTOR ALTERNATIVE 4 • I-JOIST BLOCKING PANEL TO MATCH DEPTH OF TRUSS, TJI 360, OR EQUAL • FASTEN I-JOIST TO TOP AND BOTTOM PLATES WITH 12d NAILS STAGGERED AT 4" o/c • PROVIDE WEB STIFFENERS AT INTERMEDIATE STUDS AND 2x6 HEADER AS REQUIRED BY DETAIL 7 ON S3.02 - FASTEN WITH (4)-8d NAILS

NOTE: PROVIDE SHEAR PANELING EVERY THIRD TRUSS AT SECOND FLOOR FRAMING.

2 TYPICAL SHEAR PANELING AT TRUSS BEARING CONDITIONS
3/4" = 1'-0"



SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL INFORMATION ON WALL INSULATION TYPE AND LOCATION



EXTERIOR / INTERIOR INTERSECTION

3 INSULATED STUD CORNERS
1" = 1'-0"

FASTEN SHEAR TRUSS TO SHEAR WALL STUDS - SEE TOP AND BOTTOM PLATES PLAN FOR DETAILS WITH #4 SCREWS AT 4" o/c PRE-ENGINEERED SHEAR TRUSS CENTERED WITH SHEAR WALL -DESIGN TRUSS FOR 250 #/FT. PROVIDE VERTICALS TO ALIGN WITH BEARING STUDS. DOUBLE TOP PLATES -FOR SPLICING FOR SPLICING
REQUIREMENTS SEE 15
S3.02 PROVIDE (2) SIMPSON RPS18 STRAP TIES AT NOTCHES LESS THAN 5 1/2" LONG AND AT ALL BORED HOLES LARGER THAN 40% OF PLATE WIDTH

BEARING STUDS-

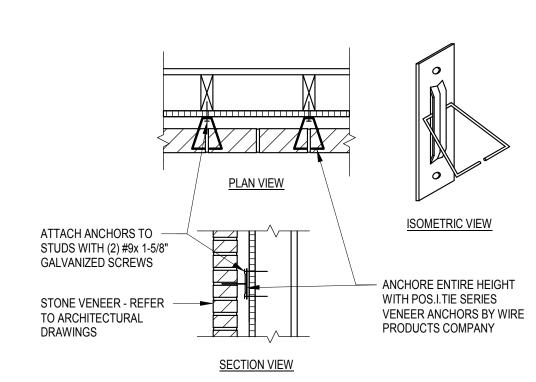
TYPICAL WINDOW -SEE ARCHITECTURAL DRAWINGS → KNEE WALL - SEE PLAN AND DETAILS EXPANSION JOINT WITH -SEALANT AND BACKER ROD PER ARCHITECTURAL DRAWINGS BRICK FACADE OR -PANEL VENNER - SEE NOTES FOR TIES AND SPACING AND DETAIL

6 TYPICAL EXPANSION JOINT AT WINDOW SILL

WINDOW JOINT SCHEDULE FLOOR GAP GROUND 3/8" LEVEL 2 1/2" . VALUES INDICATED ARE BASED SOLELY ON THE TOLERANCE OF POTENTIAL SHRINKAGE OF WOOD PLATES AT EACH LEVEL. ADDITIONAL CONSIDERATION SHALL BE ACCOUNTED FOR JOINTS CONTAINING COMPRESSIBLE JOINT FILLER, REFER TO ARCHITECTURAL DRAWINGS FOR ADDITIONAL INFORMATION. 2. DIFFERENTIAL SETTLEMENT SHALL ALSO BE CONSIDERED AT ROOF AWNINGS AND ELEVATOR

STAIR SHAFT ENTRIES. SEE GENERAL NOTES

AND DETAILS FOR ADDITIONAL INFORMATION.



TYPICAL BRICK VENEER ANCHOR TIES

VENEER ANCHOR NOTES:

- 1. STONE VENEER WALLS TALLER THAN 30'-0" BUT NOT EXCEEDING 45'-0" IN HEIGHT OR WALLS WITH A SPACING OF 1" TO 4 1/2" BETWEEN INSIDE FACE OF VENEER AND OUTSIDE FACE OF SOLID SHEATHING.
- 2. ALL TIES SHALL BE HOT DIPPED GALVANIZED BEFORE FABRICATION CONFORMING TO ASTM A-153 CLASS B2.
- 3. EMBED ALL ANCHORS INTO MORTAR JOINT 2" MINIMUM WITH AT LEAST 5/8" COVER ON OUTSIDE FACE.
- 4. SPACE ANCHORS TO PROVIDE AT LEAST ONE ANCHOR PER 2.67 SF OF WALL AREA WITH SPACING OF ANCHORS AND TIES NOT EXCEEDINIG
- 16" o/c HORIZONTAL AND 24" o/c VERTICAL. 5. PROVIDE ADDITIONAL ANCHORS AROUND ALL OPENINGS GREATER THAN 16" IN EITHER DIRECTION. SPACE ANCHORS AROUND PERIMETER OF
- OPENINGS AT 3'-0" o/c MAXIMUM AND WITHIN 12" OF OPENING. 6. SEE GENERAL NOTES FOR ADDITIONAL STONE VENEER REQUIREMENTS.

4 CONTINUOUS SHEAR TRUSS
3/4" = 1'-0"

DOUBLE TOP PLATES - SEE -

DOUBLE TOP PLATES - SEE

ATTACHMENT SCHEDULE FOR

8 TYPICAL TOP PLATE INTERSECTION
1" = 1'-0"

REQUIRED NAILING

REQUIRED NAILING

ATTACHMENT SCHEDULE FOR

LOUVERED OPENINGS IN EXTERIOR WALL

WOOD ROOF AND FLOOR TRUSS NOTES

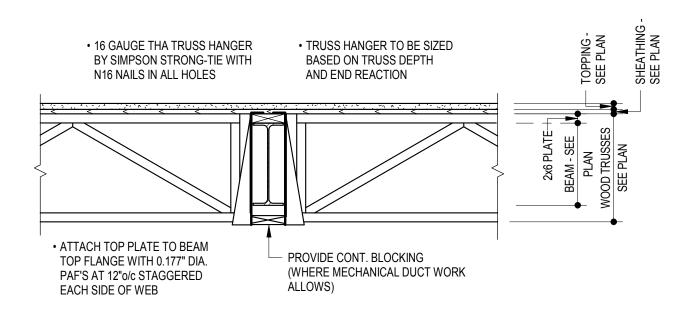
a. TRUSS MANUFACTURER IS COMPLETELY RESPONSIBLE FOR DESIGN AND FABRICATION OF ALL ROOF AND FLOOR TRUSSES, CONNECTIONS, AND BRACING. b. SHOP DRAWINGS AND CALCULATIONS SHALL BE PREPARED AND SEALED BY A STRUCTURAL ENGINEER REGISTERED IN THE PROJECT JURISDICTION. SHOP DRAWINGS AND CALCULATIONS ARE TO BE SUBMITTED TO THE STRUCTURAL ENGINEER OF RECORD AND RECEIVE APPROVAL

PRIOR TO TRUSS FABRICATION. 2. DESIGN LOADS:

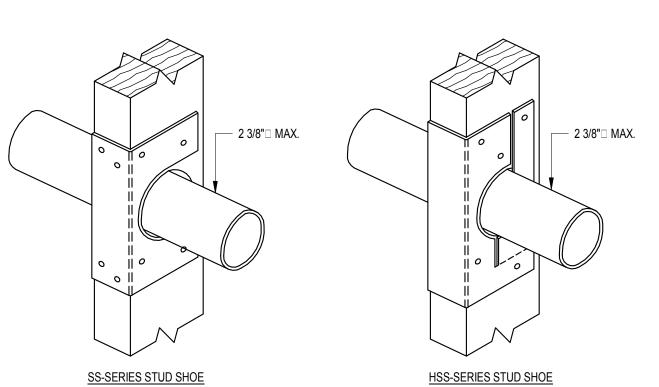
- a. FLOOR TRUSSES SHALL BE DESIGNED FOR THE APPROPRIATE FLOOR LOADINGS DESIGNATED IN THE GENERAL NOTES, ON PLAN, AND AS INDICATED IN THE PROJECT SPECIFICATIONS.
- b. FLOOR TRUSSES SHALL BE DESIGNED TO LIMIT LIVE LOAD DEFLECTION TO L/480 AND TOTAL c. AS DICTATED BY ANSI/TPI 1-2007, THE WEIGHT OF NON-BEARING PARTITION WALLS CAN BE IGNORED FOR TRUSS DESIGN PURPOSES IF THE FOLLOWING CONDITIONS ARE MET: TRUSSES ARE SPACED LESS THAN OR EQUAL TO 24"o/c.
- ii. TRUSS TOP CHORD PANEL LENGTHS ARE LESS THAN OR EQUAL TO 30". iii. LIVE LOAD IS RESULT OF A RESIDENTIAL OCCUPANCY AND NOT LESS THAN 40 PSF.
- iv. PARTITION WEIGHT IS LESS THAN OR EQUAL TO 60 PLF. v. ADDITIONAL CONSIDERATION MUST BE GIVEN FOR NON-LOAD BEARING PARTITIONS PARALLEL TO SUPPORTING TRUSSES.
- d. ROOF TRUSSES SHALL BE DESIGNED FOR THE ROOF LOADINGS DESIGNATED IN THE GENERAL NOTES AND IN THE PROJECT SPECIFICATIONS AS WELL AS ANY ADDITIONAL FLAT ROOF AND DRIFTED SNOW LOADS SHOWN ON THE ROOF FRAMING PLAN. e. ROOF TRUSSES SHALL BE DESIGNED TO LIMIT LIVE LOAD DEFLECTION TO L/360 OR 1" MAXIMUM AND TOTAL LOAD DEFLECTION OF L/240. E. COORDINATE TRUSS WEB MEMBERS WITH MECHANICAL DUCTWORK, MECHANICAL AND ELECTRICAL EQUIPMENT, AND ARCHITECTURAL LOUVERS. THE WEB SPACING AS SHOWN IS FOR GENERAL CONCEPT ONLY AND MAY NOT REFLECT THE ACTUAL WEB SPACING. ALL COST ASSOCIATED WITH COORDINATION OF ROOF TRUSS WITH MECHANICAL REQUIREMENTS SHALL

BE BORNE BY THE CONTRACTOR.

- a. PERMANENT BRACING REQUIRED FOR INDIVIDUAL TRUSS MEMBERS SHALL BE DESIGNED AND DETAILED BY THE TRUSS MANUFACTURER.
- D. ALL BRACING MEMBERS TO BE 2x4 AND ATTACHED TO TRUSS MEMBERS WITH 2-16d NAILS. c. TEMPORARY BRACING SHALL BE PROVIDED BY THE CONTRACTOR IN ACCORDANCE WITH: "BRACING WOOD TRUSSES COMMENTARY AND RECOMMENDATIONS" - TRUSS PLATE INSTITUTE PUBLICATION BWT - 76.



TYPICAL WOOD TRUSSES BEARING AT STEEL BEAM



STUD SHOE REINFORCEMENT NOTES:

- 1. STUD SHOES SHALL BE PROVIDED TO REINFORCE STUDS WHERE BORINGS DO NOT MEET THE CRITERIA SET BY DETAIL
- 2. STUD SHOES BY SIMPSON STRONG-TIE, INSTALL PER \$3.02 MANUFACTURER'S RECOMMENDATIONS. LOCATE PIPING IN 2x6 STUDS NEAR EDGE OF STUD TO PROPERLY INSTALL STUD SHOE.

2-2x4 / 2-2x6 SS3

3. ONLY (1) STUD SHOE SHALL BE PERMITTED PER STUD, IF MULTIPLE BORINGS EXIST IN SINGLE STUD, CONTACT ENGINEER.

STUD SHOE REINFORCING
3" = 1'-0"

PROJECT INFORMATION

11

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SPEXSYS

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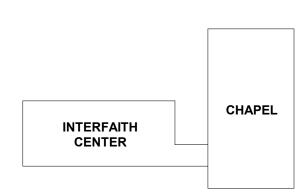
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REVISIONS DESCRIPTION **KEY PLAN**





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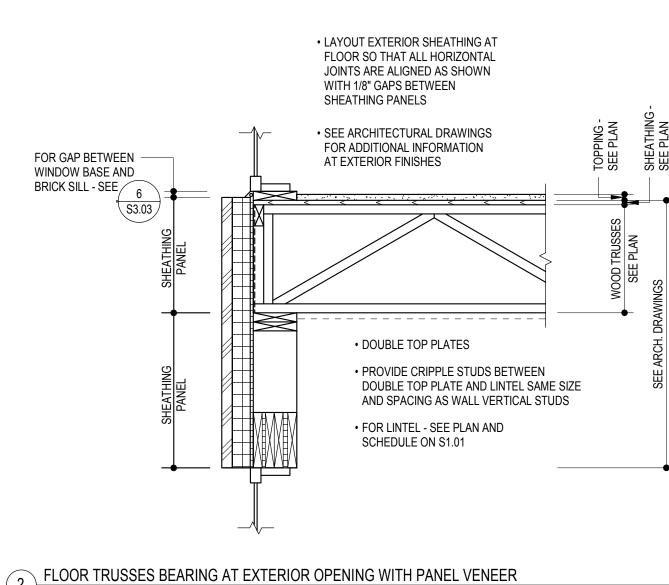
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SCALE:	As indicated	
JOB NO.:	16072	
DRAWN BY:	CRS	
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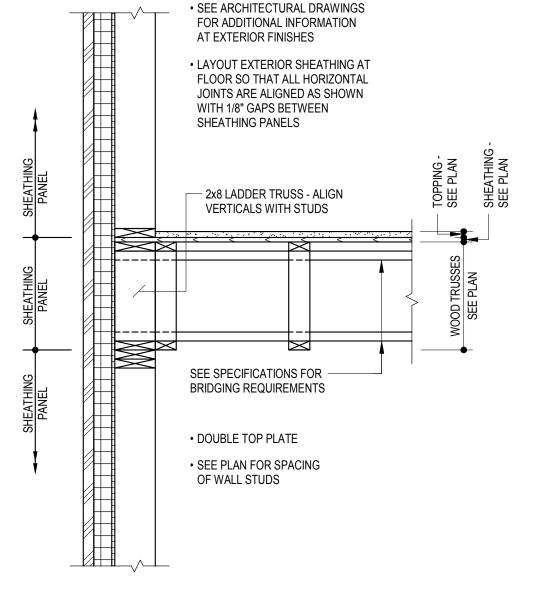
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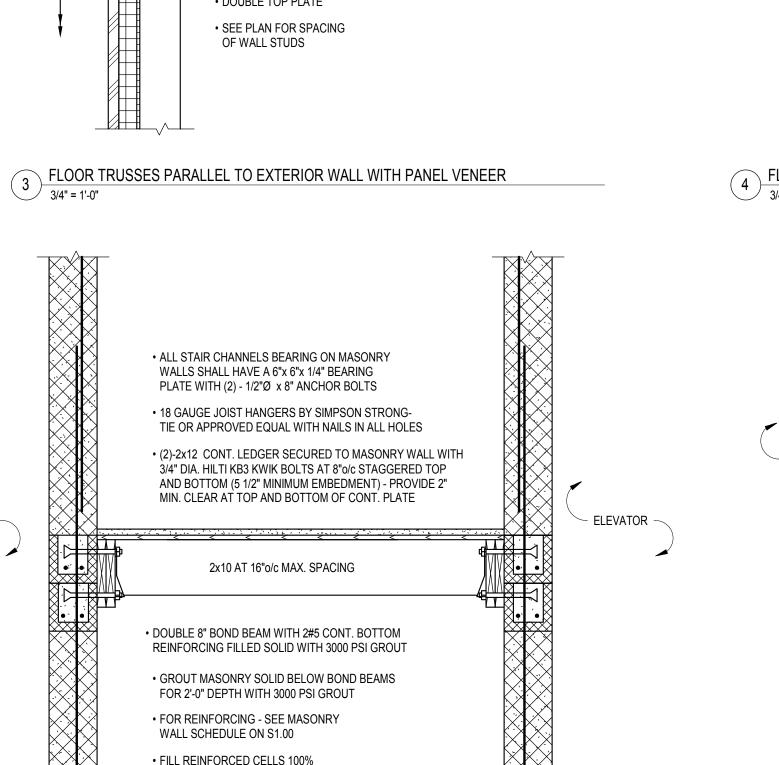
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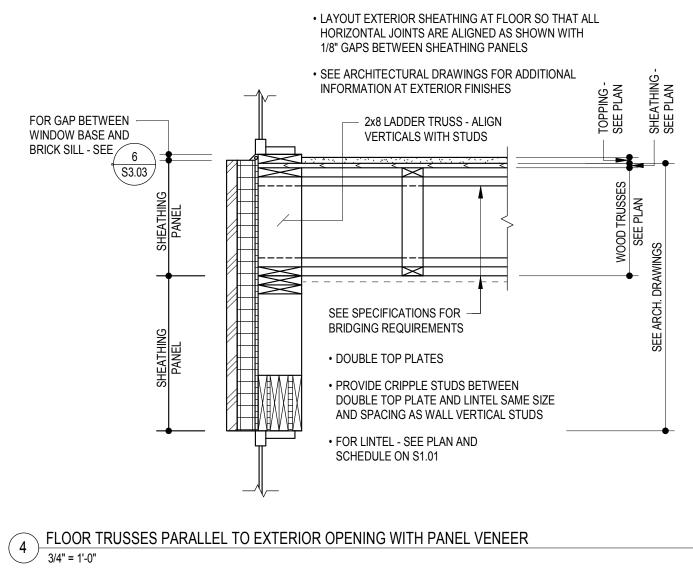
TYPICAL WOOD FRAMING **DETAILS**

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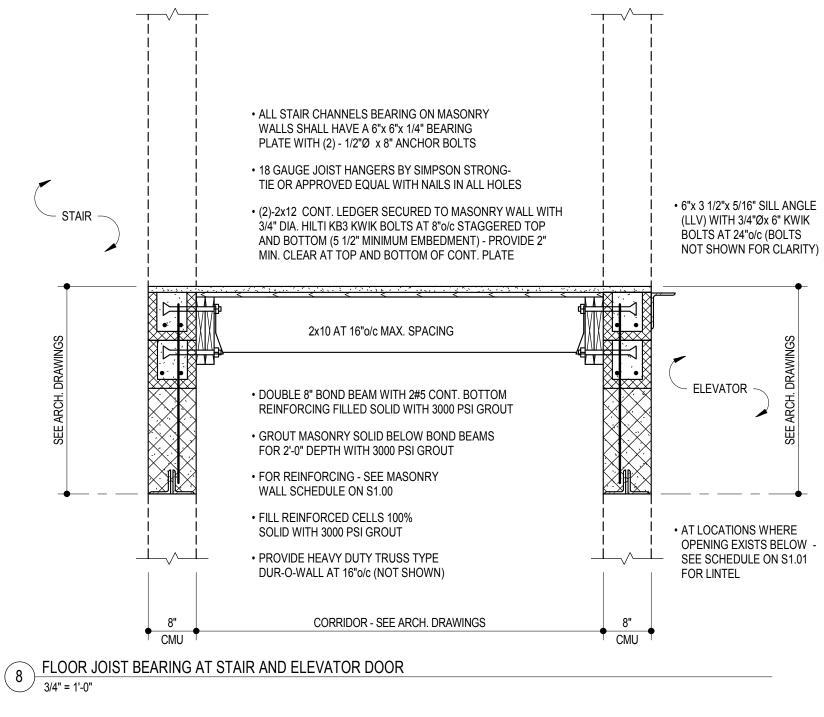


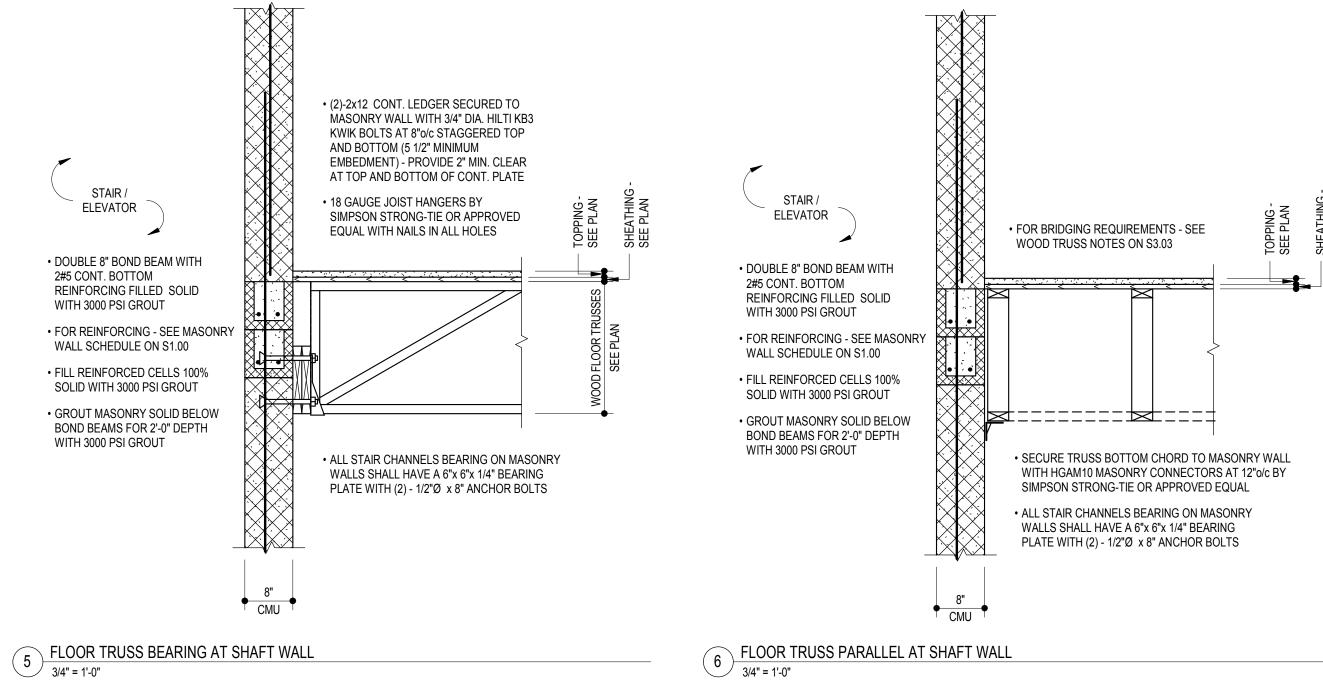


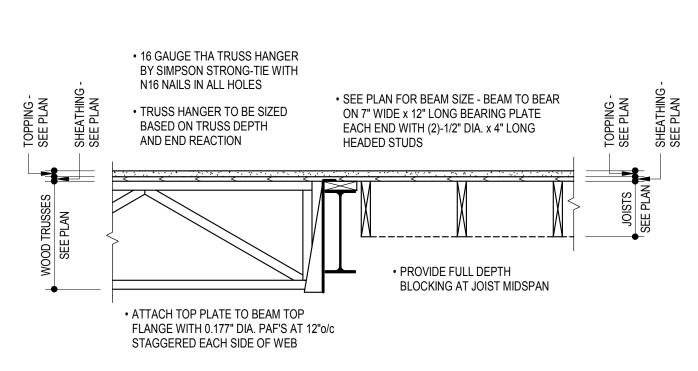




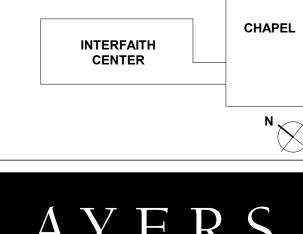
11







9 TRUSS BEARING AND JOISTS PARALLEL AT STEEL BEAM
3/4" = 1'-0"



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KEY PLAN

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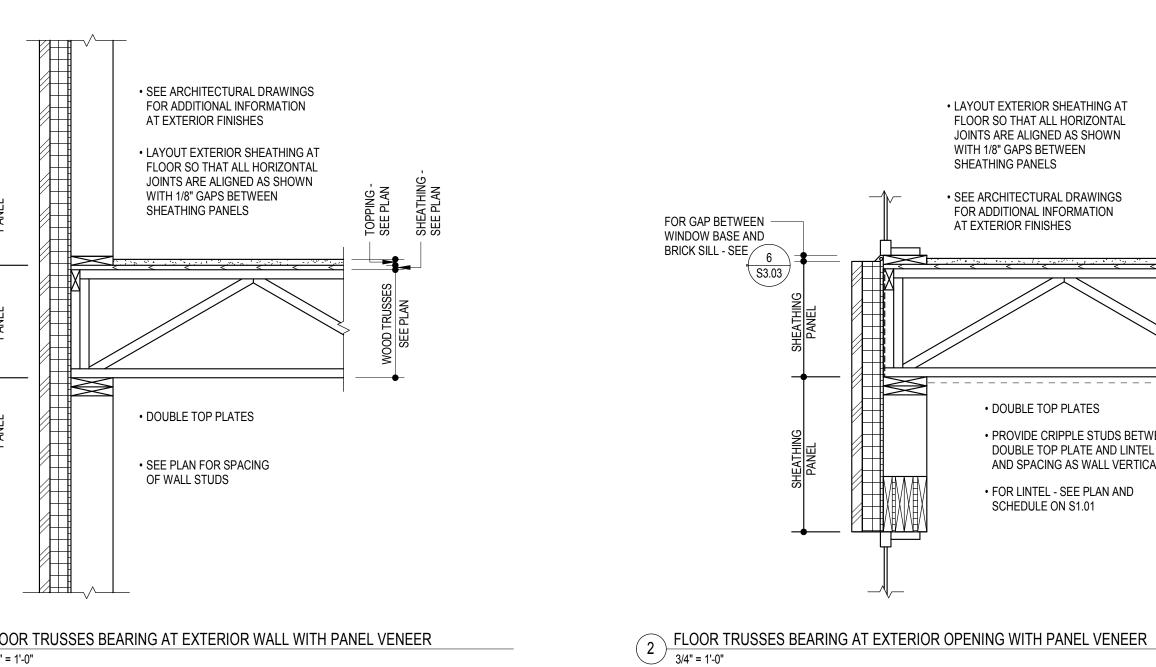
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ISSUE DATE:	04/28/17
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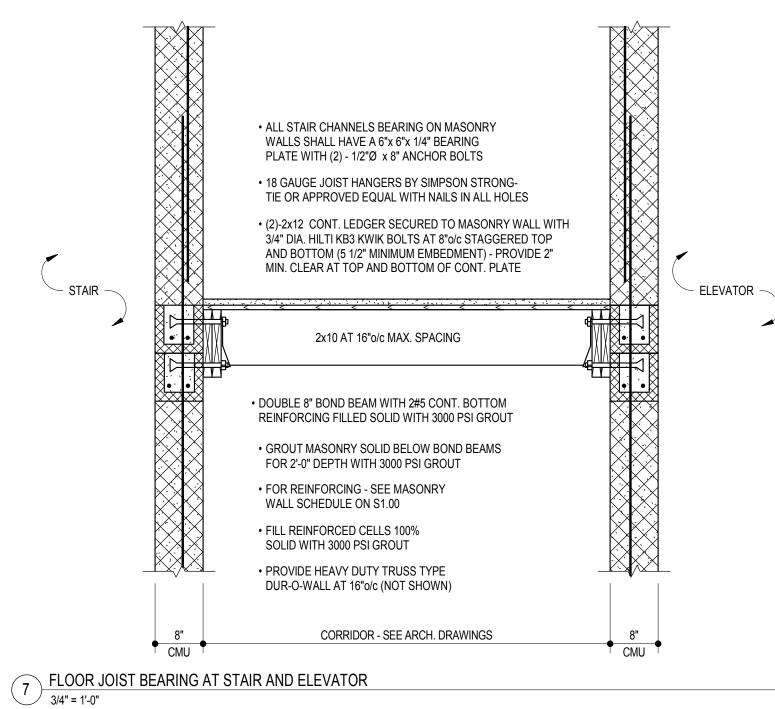
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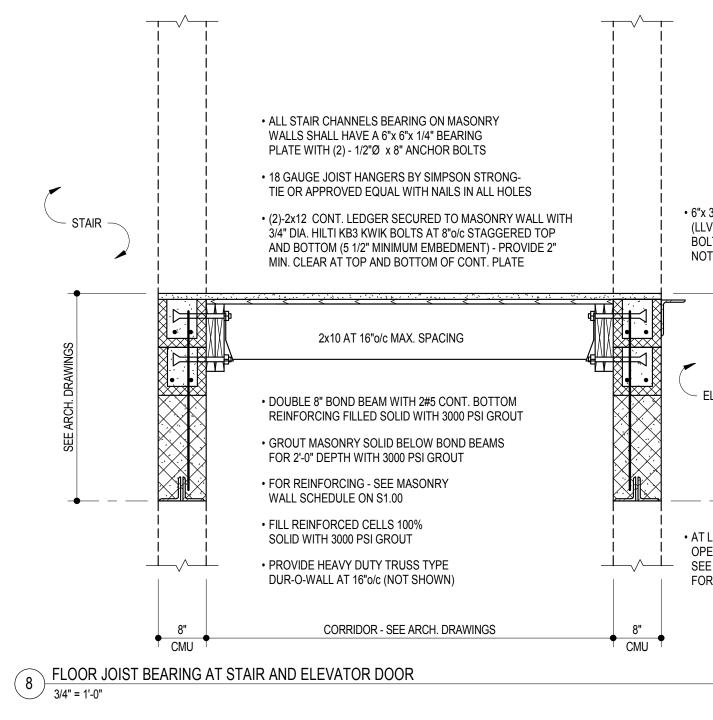
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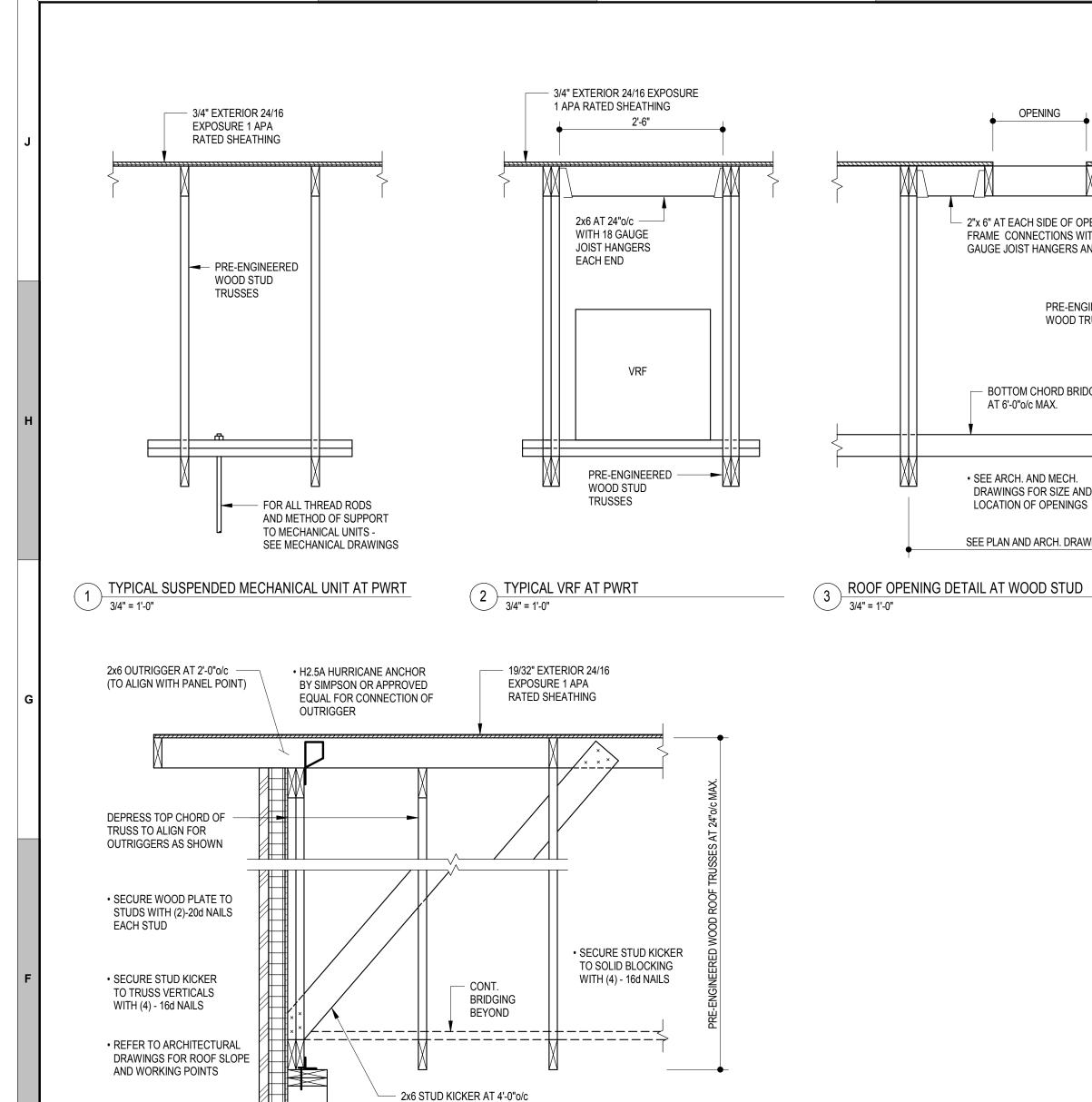
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FLOOR FRAMING DETAILS









(2) - 2x6 TOP PLATES

A34 FRAMING ANGLES AT 24"o/c

WITH 8d NAILS AS REQUIRED

• FOR EXTERIOR FACADE- SEE ARCHITECTURAL DRAWINGS

8" WOOD STUDS

AT DOUBLE GABLE END

TRUSS - PROVIDE

7 GABLE END 3/4" = 1'-0"

VERTICALS AT 24"o/c

2x4 CROSS BRACING BETWEEN ——— 19/32" EXTERIOR 24/16 — • REFER TO ARCHITECTURAL EXPOSURE 1 APA TRUSSES. CONNECT TO TRUSS WITH RATED SHEATHING 18 GAUGE JOIST HANGER (TYP.) TYP. - PRE-ENGINEERED WOOD STUD / 1/4 / ROOF TRUSSES AT 2'-0"o/c SECURE WOOD PLATE TO WIDE FLANGE • (2) - 2x6 TOP PLATE WITH H2.5A WITH 1/2" DIA. THREADED RODS AT 24"o/c HÚRRICANE ANCHOR BY • FOR ADDTIONAL INFORMATION - SEE $\left(\frac{4}{\text{S4.01}}\right)$ STAGGERED WELDED AS INDICATED. SIMPSON OR APPROVED EQUAL CL TUBE COLUMN 2'-0"o/c PRE-ENGINEERED WOOD TRUSSES AT 2'-0"o/c MAX. BEYOND ROOF TRUSS BEARING AT MOMENT FRAME

3/4" = 1'-0"

5

--- 3/4" EXTERIOR 24/16

EXPOSURE 1 APA

RATED SHEATHING

- 2"x 6" AT EACH SIDE OF OPENING -

FRAME CONNECTIONS WITH 16

GAUGE JOIST HANGERS AND 12d NAILS

BOTTOM CHORD BRIDGING

AT 6'-0"o/c MAX.

• SEE ARCH. AND MECH.

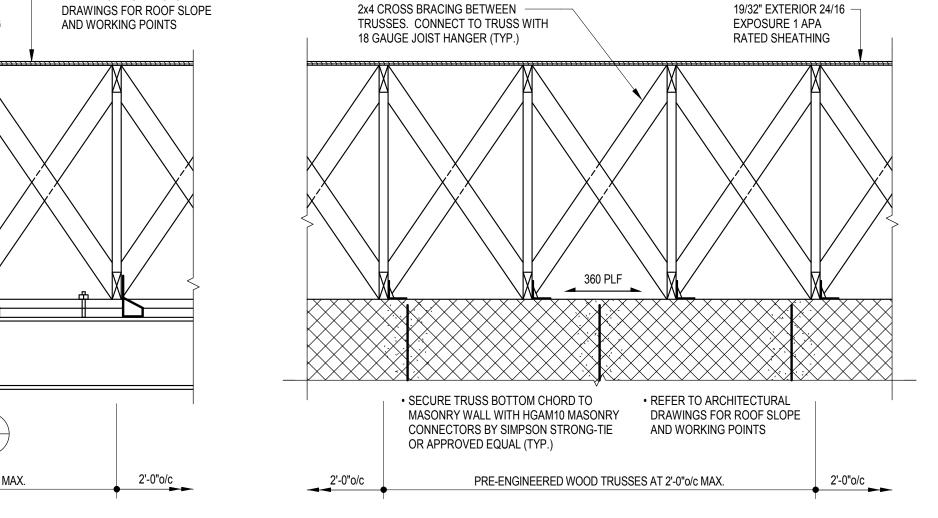
DRAWINGS FOR SIZE AND

SEE PLAN AND ARCH. DRAWINGS

LOCATION OF OPENINGS

PRE-ENGINEERED —

WOOD TRUSSES



CROSS BRACING AT MASONRY SHEAR WALL

ROOF TRUSS NOTES:

1. STRUCTURAL ROOF SHALL BE 3/4" EXTERIOR 24/16" EXPOSURE 1 APA RATED PLYWOOD SHEATHING OVER WOOD TRUSSES AT 24"o/c MAX.

2. SEE ARCHITECTURAL DRAWINGS FOR WORKING POINTS AND PITCHES OF PREFABRICATED WOOD ROOF TRUSSES.

3. CONTRACTOR TO PROVIDE ALL REQUIRED BRACING FOR WOOD TRUSSES ROOF

TRUSSES IN ACCORDANCE WITH "BUILDING COMPONENT SAFETY INFORMATION" (BCSI-2006) AS PUBLISHED BY THE TRUSS PLATE INSTITUTE. (NOT SHOWN)

4. PRE-ENGINEERED WOOD ROOF TRUSSES SHALL BE DESIGNED FOR THE FOLLOWING UNIFORMED LOADS (U.N.O.):

<u>LIVE LOAD</u> A) TOP CHORD = 30 PSF* <u>DEAD LOAD</u> A) TOP CHORD = 15 PSF B) BOTTOM CHORD = 0 PSF (TYP.) B) BOTTOM CHORD = 10 PSF*

WIND LOAD 115 MPH PER IBC 2015

FRAME, ETC. AS NECESSARY.

* IN ADDITION, ALL TRUSSES SHALL BE DESIGNED FOR SNOW DRIFT PER THE REQUIREMENTS OF IBC 2015. TRUSSES SHALL ALSO BE DESIGNED TO SUPPORT ALL MECHANICAL EQUIPMENT.

5. FIELD VERIFY ALL TRUSS DIMENSIONS PRIOR TO FABRICATION.

6. TOP CHORD OF ALL WOOD ROOF TRUSSES SHALL BE DESIGNED FOR

ALL AXIAL AND BENDING STRESSES.

7. CONTRACTOR SHALL COORDINATE TRUSS WEB MEMBERS WITH MECHANICAL DUCTWORK AND MECHANICAL AND ELECTRICAL EQUIPMENT.

8. IF SEPARATE OR PIGGY BACK TRUSSES ARE DEEMED NECESSARY BY THE

CONTRACTOR DUE TO SHIPPING AND HANDLING, ANY ADDITIONAL COST SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.

9. TRUSSES SHALL ALSO BE DESIGNED FOR WIND UPLIFT FORCES.

10.CONTRACTOR SHALL SUPPLY AND DESIGN ALL TRUSS CONNECTIONS AS REQUIRED. 11. CONTRACTOR SHALL ALSO SUPPLY AND DESIGN CONNECTIONS OF ALL TRUSS TO STRUCTURAL COLD FORMED METAL STUD WALLS, BEAMS, SLABS, STEEL

12.WHERE INDICATED ON PLAN, ROOF TRUSSES SHALL BE DESIGNED FOR AN INCREASED TOP CHORD LIVE LOAD.

13. VAULTED CEILINGS SHALL BE PROVIDED WHERE INDICATED ON PLAN. BOTTOM CHORD OF ROOF TRUSSES SHALL BE STEPPED AS REQUIRED TO FRAME THE VAULT. SEE ARCHITECTURAL DRAWINGS FOR EXACT SIZE AND LOCATION OF VAULTS.

PROJECT INFORMATION

11

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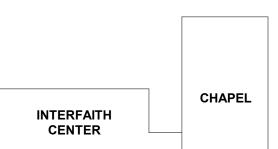
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REVISIONS REV. # DESCRIPTION

KEY PLAN



AYERS

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DRAWING INFORMATION ISSUE DATE: SCALE: 3/4" = 1'-0" JOB NO.: DRAWN BY:

PROJECT DESIGN PHASE

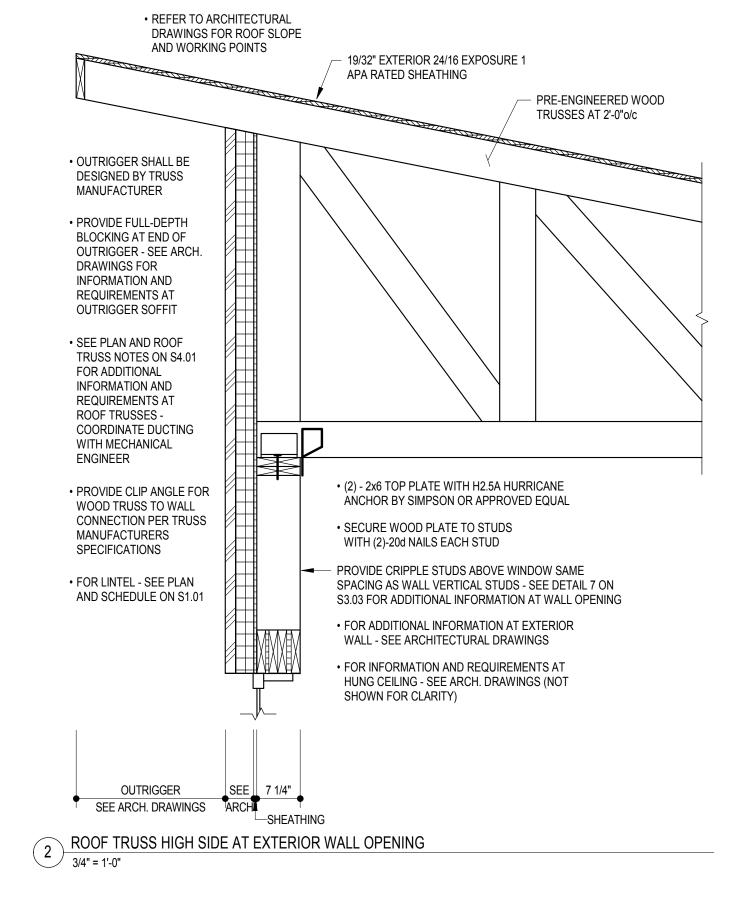
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TYPICAL ROOF DETAILS

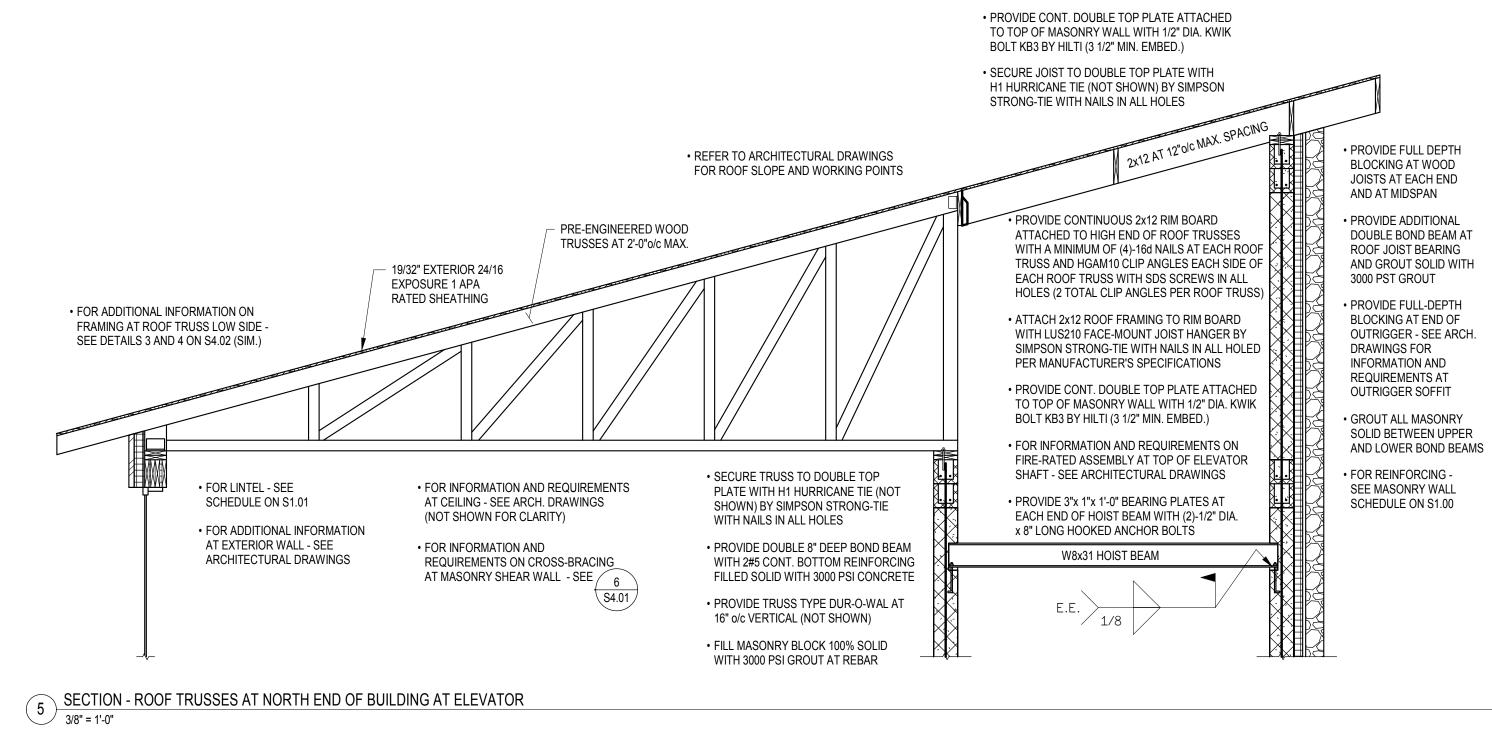
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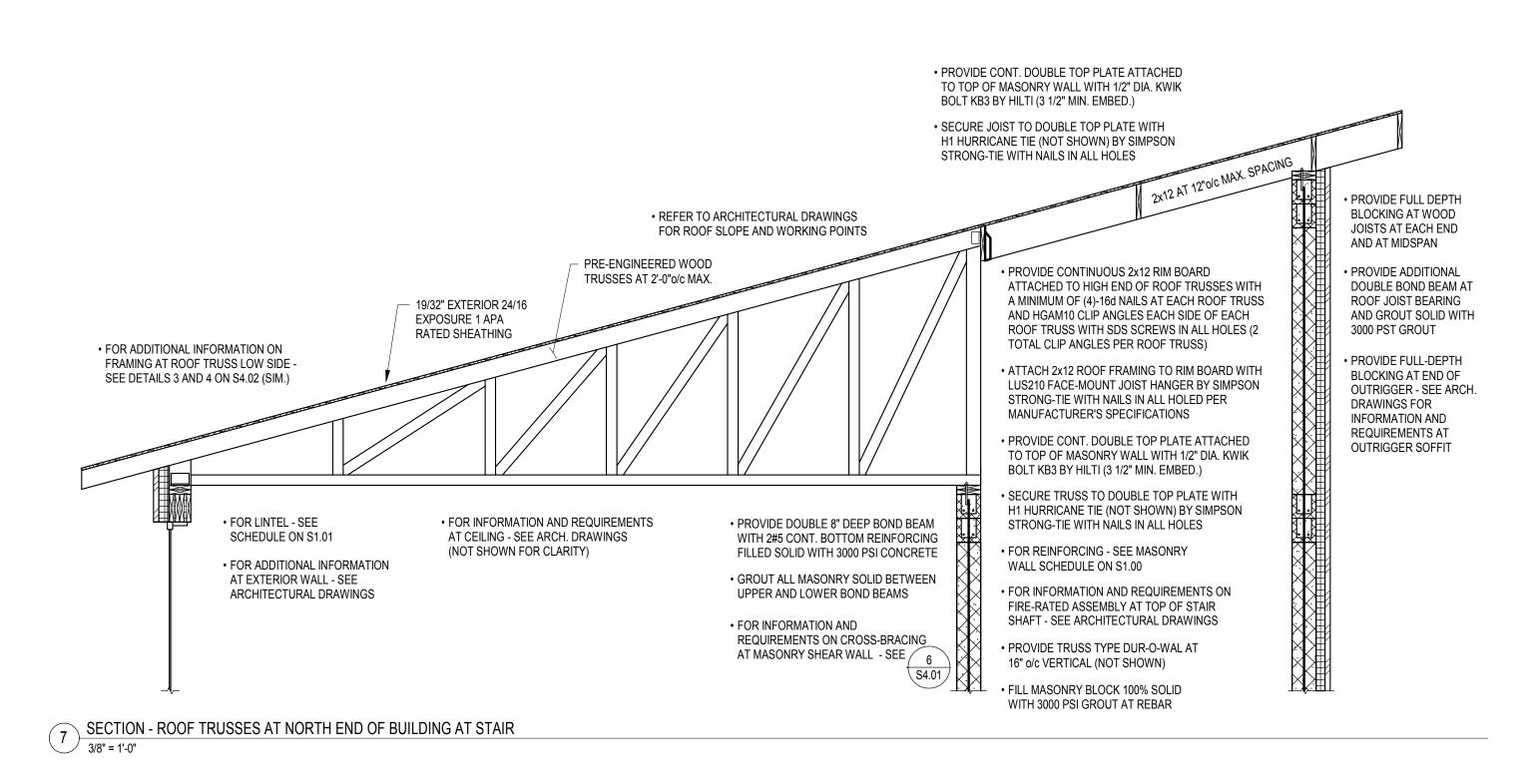
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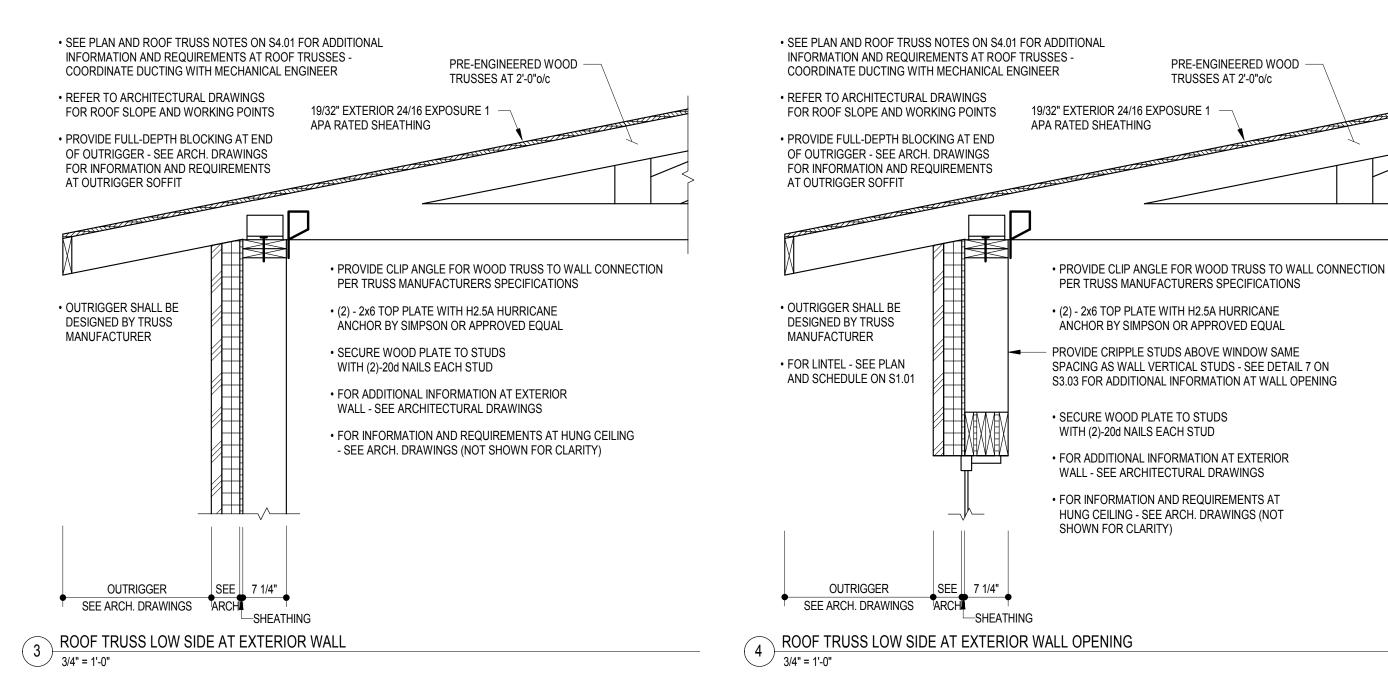
 $_{\scriptscriptstyle \setminus}$ ROOF TRUSS HIGH SIDE AT EXTERIOR WALL

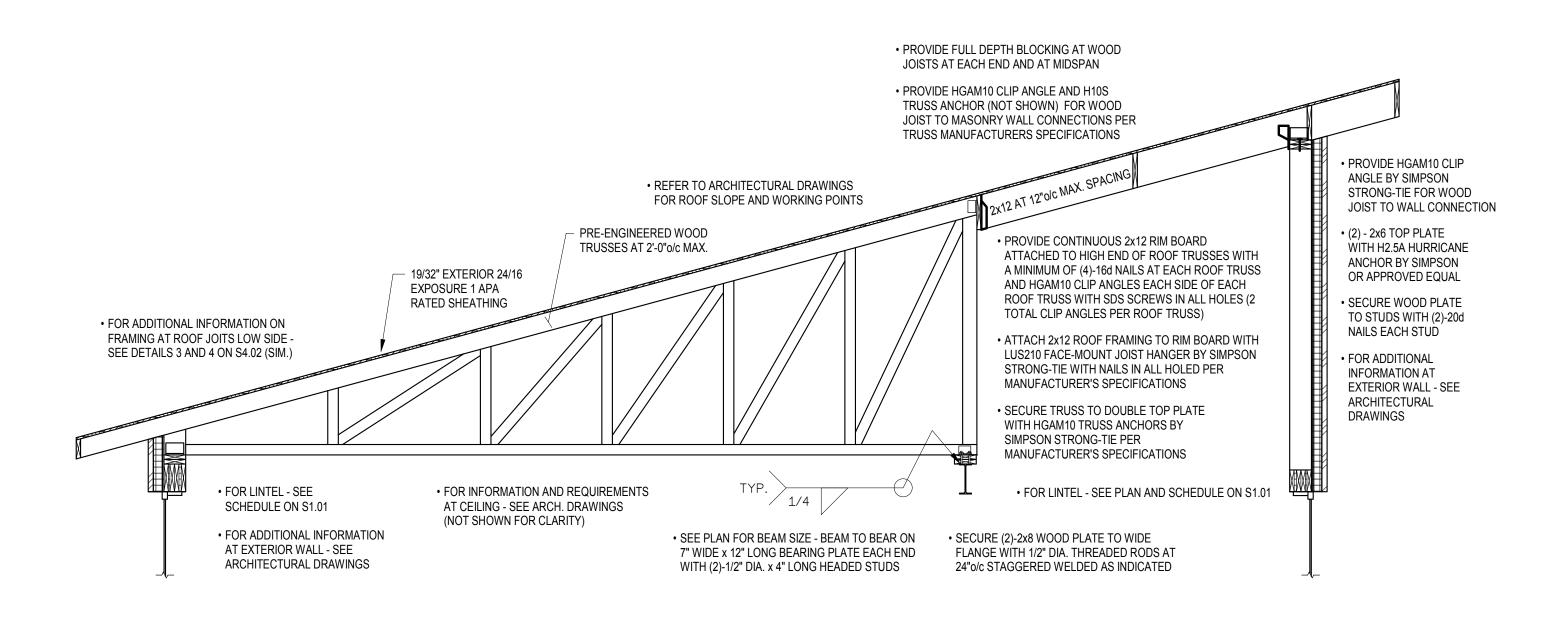


5









6 SECTION - ROOF TRUSSES AT NORTH END OF BUILDING AT BEAM

PROJECT INFORMATION

11

PRE-ENGINEERED WOOD -

TRUSSES AT 2'-0"o/c

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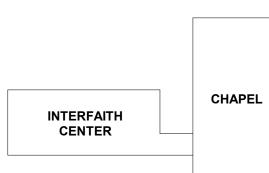
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REVISIONS DESCRIPTION **KEY PLAN**





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ISSUE DATE: SCALE: As indicated JOB NO.: DRAWN BY: PROJECT DESIGN PHASE

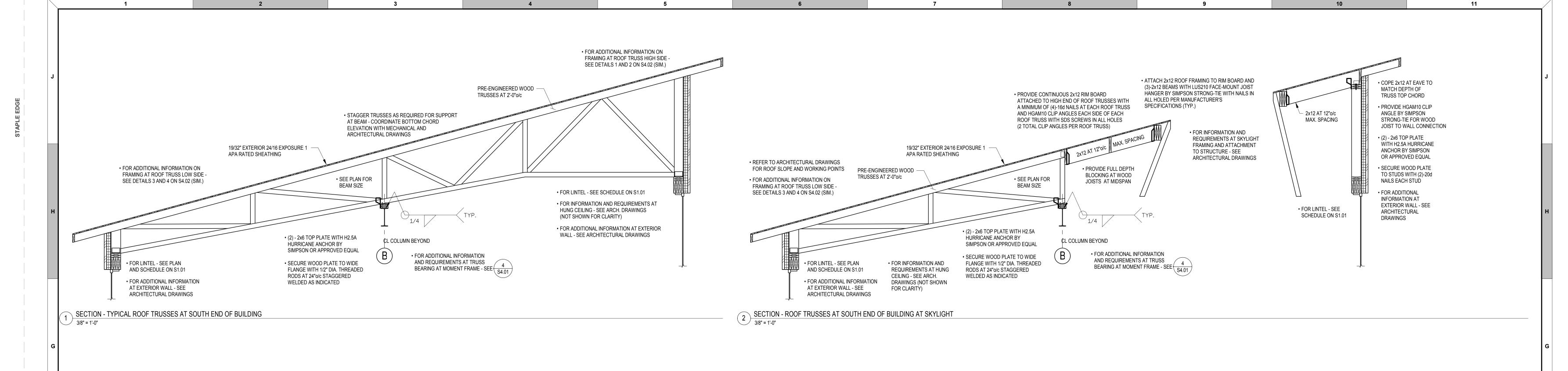
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ROOF DETAILS

DRAWING NUMBER

S4.02



PROJECT INFORMATION

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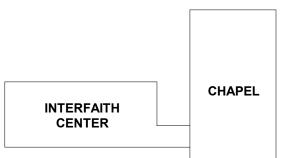
410.750.2246 www.koffel.com

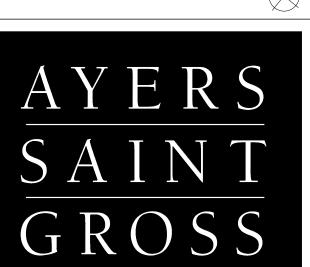
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REVISIONS					
REV.#	DESCRIPTION	DATE			

KEY PLAN





ARCHITECTS + PLANNERS

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DRAWING INFORMATION	
ISSUE DATE:	04/28/17
SCALE:	3/8" = 1'-0"
JOB NO.:	16072
DRAWN BY:	CRS
PROJECT DESIGN PHASE	<u> </u>

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DRAWING NAME

ROOF DETAILS

DRAWING NUMBER

S4.03

A. PLACEMENT AND INSTALLATION OF STEEL DECK

B. PLACEMENT AND INSTALLATION OF STEEL HEADED STUD ANCHORS. C. DOCUMENT ACCEPTANCE OR REJECTION OF STEEL ELEMENTS.

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TASK, VERIFICATION AND	FREQUENCY O	FREQUENCY OF INSPECTIONS		REFERENCE CRITERIA	
INSPECTION TASK	CONTINUOUS	PERIODIC	REF. STD.	IBC REF.	
STRUCTURAL STEEL			AISC 360	1705.6	
. INSPECTION TASKS PRIOR TO WELDING			AISC 360: N5.4		
A. WELDING PROCEDURE SPECIFICATIONS (WPSs) AVAILABLE.	X				
B. MANUFACTURER'S CERTIFICATIONS FOR WELDING CONSUMABLES AVAILABLE	X				
C. MATERIAL IDENTIFICATION (TYPE/GRADE)		X			
D. WELDER IDENTIFICATION SYSTEM.		X			
E. FIT-UP OF GROOVE WELDS (INCLUDING JOINT GEOMETRY) • JOINT PREPARATION • DIMENSIONS (ALIGNMENT, ROOF OPENINGS, ROOT FACE, BEVEL) • CLEANLINESS (CONDITION OF STEEL SURFACES) • TACKING (TACK WELD QUALITY AND LOCATION) • BACKING TYPE AND FIT (IF APPLICABLE)		X			
F. CONFIGURATION AND FINISH OF ACCESS HOLES.		X			
G. FIT-UP OF FILLET WELDS • DIMENSIONS (ALIGNMENT, GAPS AT ROOT) • CLEANLINESS (CONDITION OF STEEL SURFACE) • TACKING (TACK WELD QUALITY AND LOCATION)		X			
INSPECTION TASK DURING WELDING			AISC 360: N5.4		
A. USE OF QUALIFIED WELDERS.		X			
B. CONTROL AND HANDLING OF WELDING CONSUMABLES • PACKAGING • EXPOSURE		X			
C. NO WELDING OVER CRACKED TACK WELDS		X			
D. ENVIRONMENTAL CONDITIONS • WIND SPEED WITHIN LIMITS • PRECIPITATION AND TEMPERATURE		X			
E. WPS FOLLOWED • SETTINGS ON WELDING EQUIPMENT • TRAVEL SPEED • SELECTED WELDING MATERIALS • SHIELDING GAS TYPE / FLOW RATE • PREHEAT APPLIED • INTERPASS TEMPERATURE MAINTAINED (MIN / MAX) • PROPER POSITION (F, B, H, OH)		X			
F. WELDING TECHNIQUES • INTERPASS AND FINAL CLEANING • EACH PASS WITHIN PROFILE LIMITATIONS • EACH PASS MEETS QUALITY REQUIREMENTS		X			
I. INSPECTION TASKS AFTER WELDING			AISC 360: N5.4		
A. WELDS CLEANED		X			
B. SIZE, LENGTH AND LOCATIONS OF WELDS C. WELDS MEET VISUAL ACCEPTANCE CRITERIA • CRACK PROHIBITION • WELD / BASE-METAL FUSION • CRATER CROSS SECTION • WELD PROFILES • WELD SIZE • UNDERCUT • POROSITY	X				
D. ARC STRIKES	X				
E. K-AREA - WHEN WELDING OF DOUBLER PLATES, CONTINUITY PLATES OR STIFFENERS HAS BEEN PERFORMED IN THE K-AREA, VISUALLY INSPECT THE WEB K-AREA FOR CRACKS WITHIN 3 INCHES (75 mm) OF THE WELD	X				
F. BACKING REMOVED AND WELD TABS REMOVED (IF REQUIRED)	X				
G. REPAIR ACTIVITIES					
H. DOCUMENT ACCEPTANCE OR REJECTION OF WELDED JOINT OR MEMBER	X				
INSPECTION TASKS PRIOR TO BOLTING			AISC 360: N5.6		
A. MANUFACTURE'S CERTIFICATIONS AVAILABLE FOR FASTENER MATERIALS	X				
B. FASTENERS MARKED IN ACCORDANCE WITH ASTM REQUIREMENTS		X			
C. PROPER FASTENERS SELECTED FOR THE JOINT DETAIL (GRADE, TYPE, BOLT LENGTH IF THREADS ARE TO BE EXCLUDED FROM SHEAR PLANE)		X			
D. PROPER BOLTING PROCEDURE SELECTED FOR JOINT DETAIL		X			
E. CONNECTING ELEMENTS, INCLUDING THE APPROPRIATE FAYING SURFACE CONDITION AND HOLE PREPARATION, IF SPECIFIED, MEET APPLICABLE REQUIREMENTS.		X			
F. PRE-INSTALLATION VERIFICATION TESTING BY INSTALLATION PERSONNEL OBSERVED AND DOCUMENTED FOR FASTENER ASSEMBLIES AND METHODS USED.		X			
G. PROPER STORAGE PROVIDE FOR BOLTS, NUTS, WASHERS, AND		X			
OTHER FASTENER COMPONENTS 5. INSPECTION TASKS DURING BOLTING		Δ	AISC 360: N5.6		
A. FASTENER ASSEMBLIES, OF SUITABLE CONDITION, PLACED IN ALL		X	AISC 300: N5.0		
HOLES AND WASHERS (IF REQUIRED) ARE POSITIONED AS REQUIRED B. JOINT BROUGHT TO THE SNUG-TIGHT CONDITION PRIOR TO THE					
PRETENSIONING OPERATION		X			
C. FASTENER COMPONENT NOT TURNED BY WRENCH PREVENTED FROM ROTATING		X			
D. FASTENERS ARE PRETENSIONED IN ACCORDANCE WITH THE RCSC SPECIFICATION, PROGRESSING SYSTEMATICALLY FROM THE MOST		X			
RIGID POINT TOWARD THE FREE EDGES.					
RIGID POINT TOWARD THE FREE EDGES.	X		AISC 360: N5.6		
RIGID POINT TOWARD THE FREE EDGES. INSPECTION TASKS AFTER BOLTING: DOCUMENT ACCEPTANCE OR REJECTION	X		AISC 360: N5.6 AISC 360: N6		

TASK, VERIFICATION AND	FREQUENCY O	F INSPECTIONS	REFERENCE CRITERIA	
INSPECTION TASK	CONTINUOUS	PERIODIC	REF. STD.	IBC REF.
STEEL CONSTRUCTION OTHER				1705 2 2
THAN STRUCTURAL STEEL				1705.2.2
INSPECTION OF WELDING AND SCREWED CONNECTIONS			AISI S100-07/S1-10 AISI S200-07	
A. REINFORCING STEEL:				
VERIFICATION OF WELDABILITY OF REINFORCING STEEL OTHER THAN ASTM A 706		X		
REINFORCING STEEL RESISTING FLEXURAL AND AXIAL FORCES IN INTERMEDIATE AND SPECIAL MOMENT	X		AWS D1.4	
3.) SHEAR REINFORCEMENT	X		ACI 318: SECTION 3.5.2	
4.) OTHER REINFORCEMENT				

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	FREQUENCY OF	F INSPECTIONS	REFERENC	E CRITERIA
TASK, VERIFICATION AND INSPECTION TASK	CONTINUOUS	PERIODIC	REF. STD.	IBC REF.
CONCRETE CONSTRUCTION			ACI 318	1705.3
INSPECTION OF REINFORCING STEEL, INCLUDING PRESTRESSING TENDONS, AND PLACEMENT		X	ACI 318: 2.4, 7.1-7.7	1910.4
2. INSPECTION OF REINFORCING STEEL WELDING IN ACCORDANCE WITH TABLE 1705.2.2, ITEM 2 (IBC)			AWS D1.4 ACI 318: 3.5.2	
3. INSPECTION OF ANCHORS CAST IN CONCRETE WHERE ALLOWABLE LOADS HAVE BEEN INCREASED OR WHERE STRENGTH DESIGN IS USED		X	ACI 318: 8.1.3, 21.2.8	1908.5, 1909.1
4. INSPECTION OF ANCHORS POST INSTALLED IN HARDENED CONCRETE.		X	ACI 318: 3.8.6, 8.1.3, 21.2.8	1909.1
5. VERIFY USE OF REQUIRED DESIGN MIX.		X	ACI 318: CH. 4, 5.2-5.4	1904.2, 1910.2, 1910.3
6. AT THIS TIME FRESH CONCRETE IS SAMPLED TO FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF CONCRETE	X		ASTM C172 ASTM C 31 ACI 318: 5.6, 5.8	1910.10
7. INSPECTION OF CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATIONS TECHNIQUES	X		ACI 318: 5.9, 5.10	1910.6, 1910.7, 1910.8
8. INSPECTION FOR MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES		X	ACI 318: 5.11, 5.13	1910.9
9. INSPECTION OF PRESTRESSED CONCRETE:				
A. APPLICATION OF PRESTRESSING FORCES.	X			
B. GROUTING OF BOUNDED PRESTRESSING TENDONS IN THE SEISMIC FORCE-RESISTING SYSTEM	X		ACI 318: 18.18.4, 18.20	
10.ERECTION OF PRECAST CONCRETE MEMBERS.		X	ACI 318: CH. 16	
11. VERIFICATION OF IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS		X	ACI 318: 6.2	
12.INSPECT FORMWORK FOR SHAPE LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED		X	ACI 318: 6.1.1	

	FREQUENCY O	F INSPECTIONS	R	EFERENCE CRITE	RIA
TASK, VERIFICATION AND INSPECTION TASK	CONTINUOUS	PERIODIC	IBC REF.	TMS 402 ACI 530 ASCE 5	TMS 602 ACI 530.1 ASCE 6
MASONRY CONSTRUCTION LEVEL B			1705.4		
COMPLIANCE WITH REQUIRED INSPECTION PROVISIONS OF THE CONSTR'N DOCUMENTS AND THE APPROVED SUBMITTALS SHALL BE VERIFIED		X			ART. 1.5
2. VERIFICATION OF f'm AND faac PRIOR TO CONSTRUCTION EXCEPT WHERE SPECIFICALLY EXEMPTED BY THE CODE		X			ART. 1.4 B
3. VERIFICATION OF SLUMP FLOW AND VISUAL STABILITY INDEX (VSI) AS DELIVERED TO THE SITE FOR SELF-CONSOLIDATING GROUT	X				ART. 1.5 B.1, B.3
4. AS MASONRY CONSTRUCTION BEGINS, THE FOLLOWING SHALL BE VERIFIED TO ENSURE COMPLIANCE:					
A. PROPORTIONS OF SITE-PREPARED MORTAR.		X			ART. 2.1, 2.6 A
B. CONSTRUCTION OF MORTAR JOINTS		X			ART. 3.3 B
C. GRADE AND SIZE OF PRESTRESSING TENDONS AND ANCHORAGES		X			ART. 2.4 B, 2.4 H
D. LOCATION OF REINFORCEMENT, CONNECTORS, PRESTRESSING TENDONS AND ANCHORAGES		X			ART. 3.4, 3.6 A
E. PRESTRESSING TECHNIQUE		X			ART. 3.6 B
F. PROPERTIES OF THIN-BED MORTAR FOR AAC MASONRY	X (FIRST 5000 SQ. FT.)	X (AFTER FIRST 5000 SQ.			ART. 2.1 C
5. PRIOR TO GROUTING, THE FOLLOWING SHALL BE VERIFIED FOR COMPLIANCE:	· · ·	FT.)			
A. GROUT SPACE IS CLEAN		X			ART. 3.2 D, 3.2 F
B. GRADE, TYPE AND SIZE OF REINFORCEMENT AND ANCHOR BOLTS AND PRESTRESSING TENDONS AND ANCHORAGES		X		SEC. 1.16	ART 2.4, 3.4
C. PLACEMENT OF REINFORCEMENT AND CONNECTORS AND PRESTRESSING TENDON AND ANCHORAGES		X		SEC. 1.16	ART. 3.4, 3.2 E, 3.6 A
D. PROPORTIONS OF SITE-PREPARED GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS		X			ART. 2.6 B, 2.4 G.1.b
E. CONSTRUCTION OF MORTAR JOINTS		X			ART. 3.3 B
3. DURING CONSTRUCTION THE FOLLOWING SHALL BE VERIFIED:					
A. SIZE AND LOCATION OF STRUCTURAL ELEMENTS		X			ART. 3.3 F
B. TYPE, SIZE AND LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES OR OTHER CONSTRUCTION		X		SEC. 1.16.4.3, 1.17.1	
C. WELDING OF REINFORCING BARS	X			SEC. 2.1.8.7.2, 3.3.3.4(c), 8.3.3.4(b)	
D. PREPARATION, CONSTRUCTION AND PROTECTION OF MASONRY DURING COLD WEATHER (TEMPERATURE BELOW 40°F) OR HOT WEATHER (TEMPERATURE ABOVE 90°F)		X	SEC. 2104.3, 2104.4		ART. 1.8 C, 1.8 D
E. APPLICATION AND MEASUREMENT OF PRESTRESSING FORCE	X				ART. 3.6 B
F. PLACEMENT OF GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS IS IN COMPLIANCE	X				ART. 3.5, 3.6 C
G. PLACEMENT OF AAC MASONRY UNITS AND CONSTRUCTION OF THIN-BED MORTAR JOINTS	X (FIRST 5000 SQ. FT.)	X (AFTER FIRST 5000 SQ.			ART. 3.3 B.8
7. OBSERVE PREPARATION OF GROUT SPECIMENS, MORTAR SPECIMENS AND/OR PRISMS		FT.)	SEC. 2105.2.2, 2105.3		ART 1.4 B.2a.3, 1.4 B.2.l 1.4 B.2.c.3, 1.4 B.3, 1.4 l
B. VERTICAL MASONRY FOUNDATION ELEMENTS		X	1705.4		

TASK, VERIFICATION AND	FREQUENCY O	F INSPECTIONS	REFERENCE CRITERIA	
INSPECTION TASK	CONTINUOUS	PERIODIC	REF. STD.	IBC REF.
SOILS				1705.6
CONTROLLED FILL PLACED UNDER SITE PERMIT				
2. CONTROLLED FILL PLACED UNDER THIS BUILDING PERMIT	X			
3. VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY		X		
4. VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL		X		
5. PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS		X		
6. VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESS DURING PLACEMENT AND COMPACTION OF COMPACTED FILL	X			
7. PRIOR TO PLACEMENT OF COMPACTED FILL, OBSERVE SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY		X		
8. VERIFY INSTALLATION OF DRAIN TILE (GRAVITY/MECHANICAL)		X		

SPECIAL INSPECTIONS NOTES:

- 5. THE SPECIAL INSPECTOR SHALL SUBMIT INSPECTION REPORTS TO THE CONTRACTOR, ARCHITECT, OWNER AND MORABITO CONSULTANTS. REPORTS SHALL DOCUMENT REQUIRED INSPECTIONS AND CORRECTIONS OF ANY DISCREPANCIES. REPORTS SHALL BE PROVIDED AT

1. THE OWNER WILL ENGAGE (SEE CONTRACT REQUIREMENTS) THE SERVICES OF ONE OR MORE SPECIAL INSPECTORS TO PROVIDE INSPECTIONS DURING CONSTRUCTION ON WORK INDICATED IN THE SCHEDULE OF SPECIAL INSPECTIONS. IN ACCORDANCE WITH THE PROVISIONS OF CHAPTER 17 OF THE INTERNATIONAL BUILDING CODE.

- 2. SPECIAL INSPECTIONS AND TESTING SHALL BE PERFORMED ON A CONTINUOUS OR PERIODIC FREQUENCY AS NOTED IN THE SCHEDULE.
- 3. REFER TO THE GENERAL NOTES AND SPECIFICATIONS FOR ADDITIONAL INSPECTION AND TESTING REQUIREMENTS.
- 4. DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF DISCREPANCIES ARE NOT CORRECTED, CONTACT MORABITO CONSULTANTS PRIOR TO COMPLETION OF THAT PHASE OF THE WORK.
- INTERVALS CONVEYING THE PROGRESS OF CONSTRUCTION.

PROJECT INFORMATION

11

PROJECT TEAM

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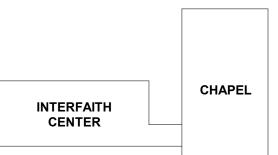
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KEY PLAN





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LIST OF SPECIAL

INSPECTIONS DRAWING NUMBER

S5.01

PROJECT INFORMATION **GOLDSMITH INTERFAITH** GOUCHER COLLEGE 1021 DULANEY VALLEY RD BALTIMORE MD 21204

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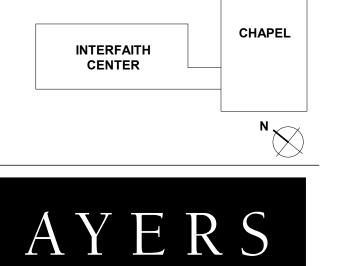
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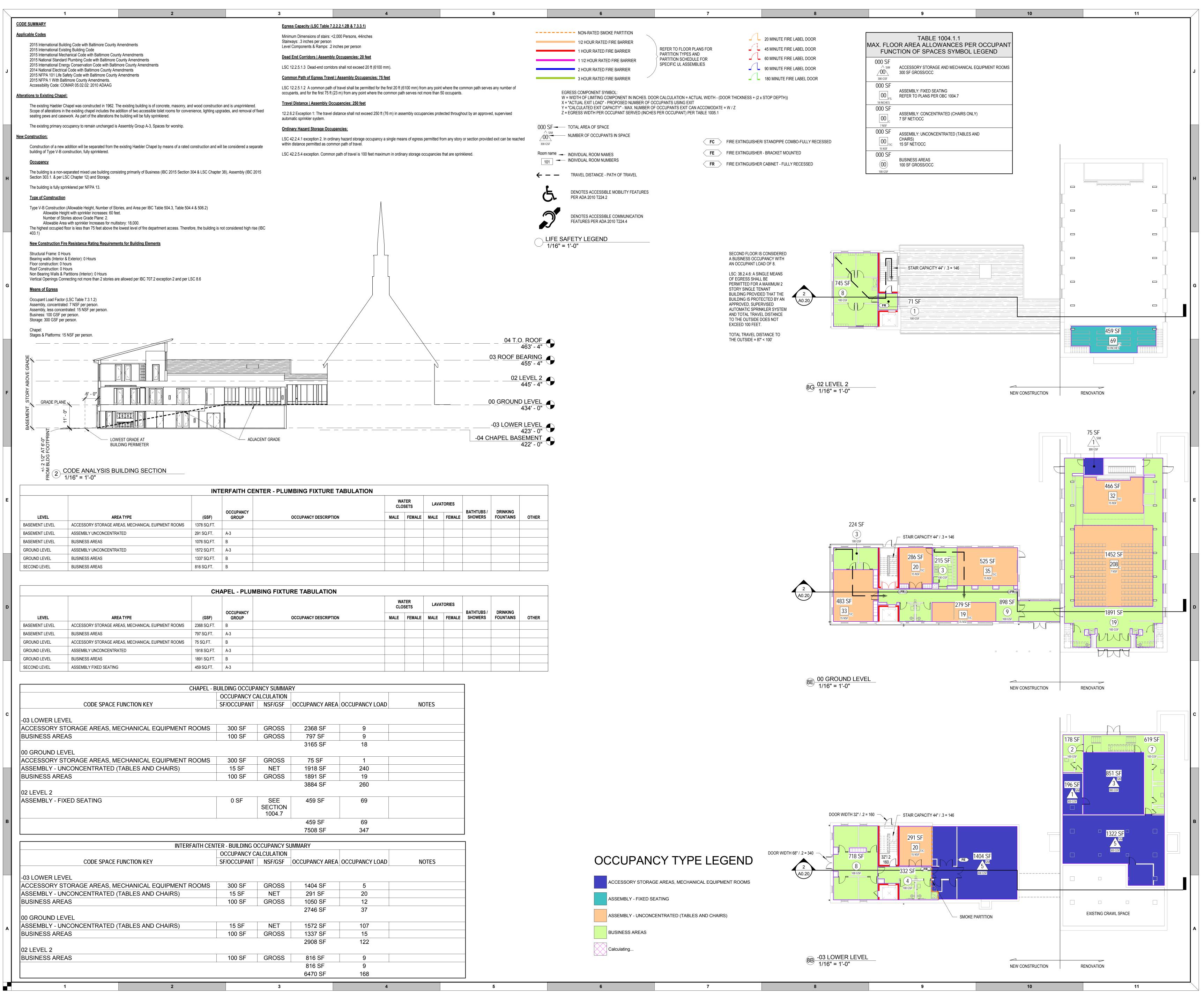
DOCUMENTS

DRAWING NAME

ABBREVIATIONS AND SYMBOLS

DRAWING NUMBER





PROJECT INFORMATION

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REVISIONS DESCRIPTION

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CODE CONSULTANT

KOFFEL ASSOCIATES

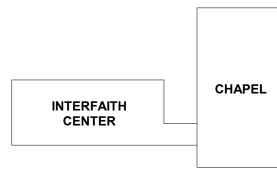
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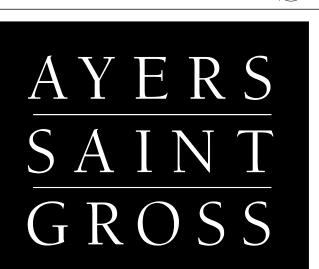
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REV. # DESCRIPTION DATE





ARCHITECTS + PLANNERS

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DRAWING INFO	RMATION
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PROJECT DESIG	GN PHASE

50% CONSTRUCTION DOCUMENTS

DRAWING NAME

ARCHITECTURAL SITE PLAN

A1.00

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DEMOLITION GENERAL NOTES

ETC. AS REQUIRED.

1. DEMOLITION PLANS ARE PROVIDED FOR THE SOLE BENEFIT OF THE CONTRACTOR AND SHALL NOT DETERMINE THE TOTAL SCOPE OR LIMITS OF DEMOLITION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PERFORMING ALL DEMOLITION AS REQUIRED TO ACHIEVE THE NEW CONSTRUCTION AS DESCRIBED IN ALL NEW CONSTRUCTION DOCUMENTS AT NO ADDITIONAL COST TO THE OWNER.

2. ALL DEMOLITION WORK SHALL BE DONE IN ACCORDANCE WITH STATE AND LOCAL REGULATIONS AND CODES. CONFORM TO ALL APPLICABLE ORDINANCES REGARDING SAFETY, ENVIRONMENTAL PROTECTION ETC. RELATED TO THE WORK.

3. WHERE DEMOLITION IS REQUIRED. BUT NOT INDICATED FOR INSTALLATION OF NEW WORK INDICATED. DEMOLITION SHALL BE COMPLETED AS IF INDICATED, AT NO ADDITIONAL COST TO THE OWNER.

4. ALL EXISTING CONDITIONS AND DIMENSIONS INDICATED ON DRAWINGS ARE TO BE VERIFIED IN THE FIELD PRIOR TO PROCEEDING WITH ANY WORK.

5. ITEMS TO BE REMOVED AND DISCARDED ARE FIRST TO BE REVIEWED WITH OWNER FOR SALVAGE. ALL OTHER ITEMS ARE TO BE DISPOSED OF IN A MANNER ACCEPTABLE TO OWNER AND LOCAL JURISDICTION

AND SATISFY LEED PROJECT GOALS. 6. PROTECT THE OCCUPANTS, PREMISES & UTILITIES AND PROVIDE DUST BARRIERS SAFETY BARRICADES

7. CONTRACTOR TO PROTECT ALL EXISTING ITEMS TO REMAIN INCLUDING, BUT NOT LIMITED TO, ALL GLASS AND FINISHES DURING DEMOLITION AND CONSTRUCTION. REPAIR TO MATCH EXISTING WORK.

8. WHERE WORK IS NOTED TO BE REMOVED, ADJACENT WALL CEILINGS, FLOORS AND FINISHES SHALL BE PATCHED AND LEVELED, AS REQUIRED, TO BLEND TOGETHER TO MATCH EXISTING. ALL SELECTIVE DEMOLITION WORK ADJACENT REMAINING CONSTRUCTION MUST BE DONE IN A WORKMAN LIKE MANNER IN PREPARATION FOR THE NEW WORK.

9. PATCH SURFACES TO REMAIN SUCH THAT COMPLETED REPAIR IS INDISTINGUISHABLE FROM ADJACENT WORK. PAINT BEYOND REPAIRED AREA TO NEXT CORNER AT FLOOR, WALL, AND CEILING.

10. SEAL EXISTING PIPES AND DUCT INTAKES (NOTED TO REMAIN) AS REQUIRED TO AVOID SYSTEM CONTAMINATION FROM CONSTRUCTION DUST.

11. DO NOT CUT OR REMOVE CONSTRUCTION WHICH MIGHT WEAKEN THE STRUCTURAL INTEGRITY OR STRENGTH OF THE STRUCTURAL FRAMING OR SUPPORT SYSTEMS SCHEDULED TO REMAIN.

12. ALL LIFE SAFETY SYSTEMS SHALL REMAIN ACTIVE DURING DEMOLITION AND CONSTRUCTION
OPERATIONS. THE BUILDING SHALL BE MAINTAINED AND LEFT IN A SAFE CONDITION. ALL HAZARDS AND
UNSAFE CONDITIONS SHALL BE IDENTIFIED AND CONTRACTOR SHALL PROVIDE PROPER NOTIFICATION TO

13. THIS IS AN OCCUPIED SITE, THE BUILDING SHALL BE LEFT BROOM CLEAN AND DRY AT THE END OF EACH WORK DAY.

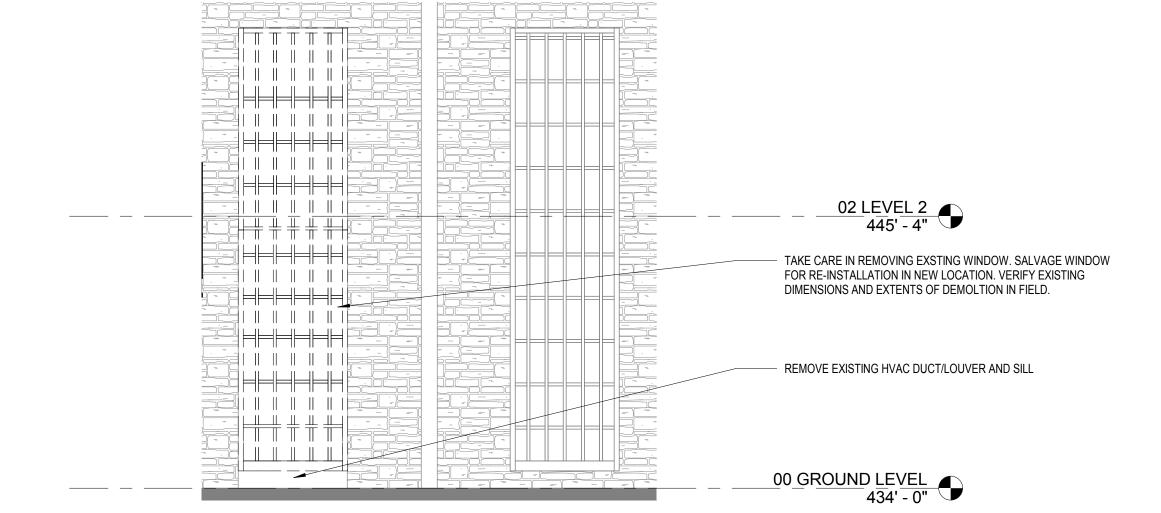
14. RESTORE ANY SURFACES OF FINISHES WHICH ARE SCRATCHED, MARRED OR OTHERWISE DAMAGED BE THE INSTALLATION, MOVEMENT OR REMOVAL OF ALL EQUIPMENT ASSOCIATED WITH DEMOLITION PROCEDURES. (EG. SCAFFOLDING, CONTAINERS, ETC.).

15. CONTRACTOR SHALL REMOVE ALL MISC. CASEWORK ALONG WITH ASSOCIATED PLUMBING, GAS, POWER AND AIR, FIXTURES AND LINES BASES, SUPPORTS, AND ALL ABANDONED FASTENERS IN THEIR ENTIRETY. ALL CASEWORK IS TO BE CAREFULLY REMOVED TO ALLOW THE OWNER TO SALVAGE ANY REUSABLE EQUIPMENT. SEE MECHANICAL / ELECTRICAL / PLUMBING DOCUMENTS FOR RELATED WORK.

16. REMOVE ALL DEMOLITION MATERIAL AND DEBRIS FROM SITE.

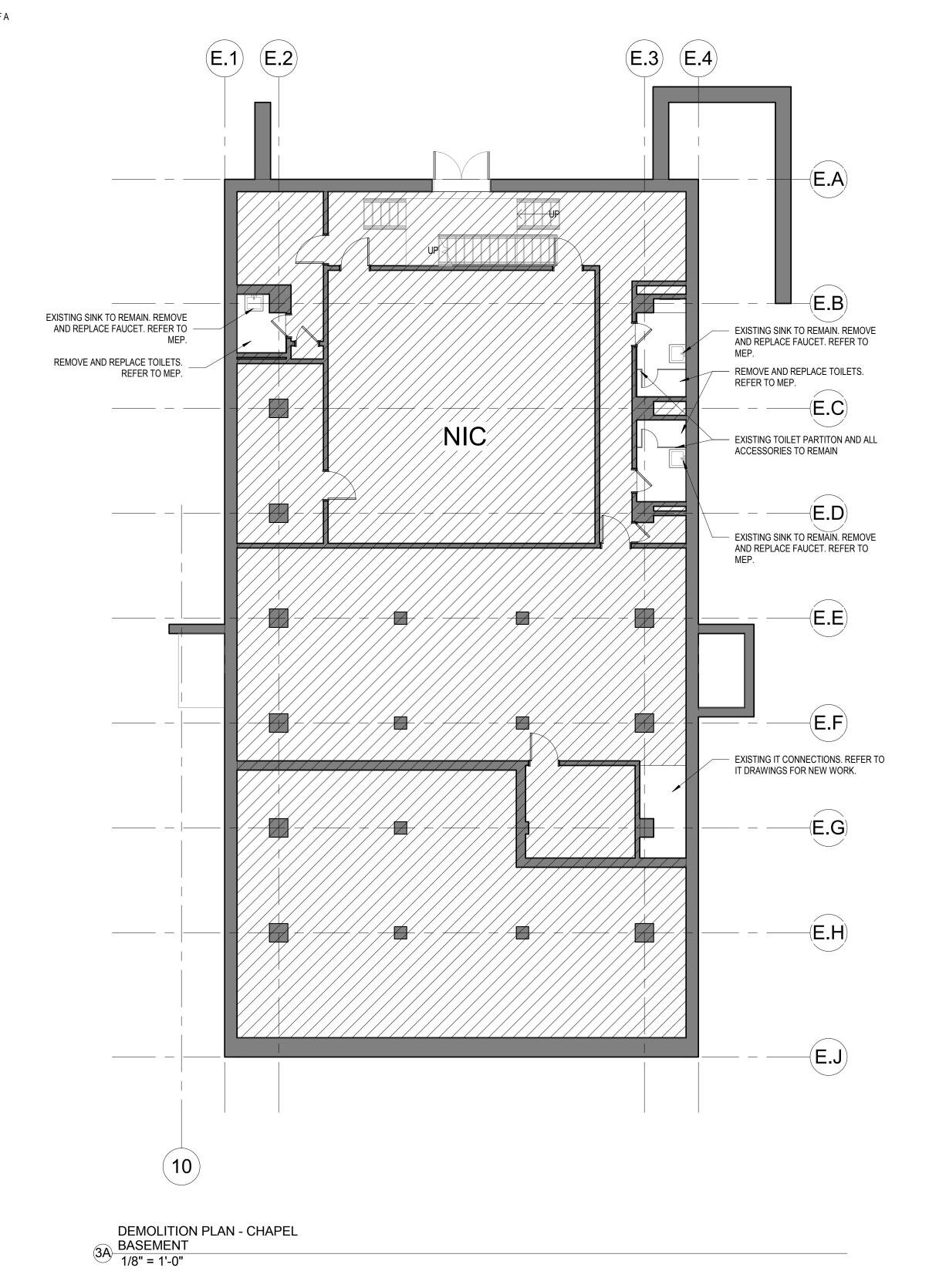
17. WHERE EXISTING MASONRY ABUTS INTO EXISTING MASONRY AND ONE WALL IS DEMOLISHED, THE CONTRACTOR WILL REPAIR THE INTERSECTION BY TOOTHING-IN NEW MASONRY TO MATCH EXISTING. 18. CONTRACTOR SHALL REMOVE ABANDONED PLUMBING FIXTURES, CAP DRAINS AND RELATED PIPING, AND CAP ALL UNUSED PIPING BEHIND WALL, ABANDONED FLOOR DRAINS TO BE CAPPED AND SEALED. U.N.O. SEE MECHANICAL / ELECTRICAL / PLUMBING DOCUMENTS.

19. DO NOT REMOVE EXISTING COLUMNS WITHOUT CROSS REFERENCING STRUCTURAL PLANS. IF A DISCREPANCY EXIST BETWEEN PLANS CONTACT ARCHITECT PRIOR REMOVAL.



REMOVE EXISTING CLOSET.

 $7G \frac{\text{WINDOW}}{1/4" = 1'-0"}$



5

EXISTING WOOD SCREENS TO REMAIN. PROTECT DURING DEMOLITION AND NEW WORK. TAKE CARE IN REMOVING EXISTING STAIRS. SALVAGE AND RETURN TO OWNER. EXISTING ORGAN, ORGAN CONSOLE AND ALL PIPING AND PIT FOR ORGAN CONSOLE TO REMAIN TAKE CARE IN REMOVING CORK TILE FLOORING FROM PLATFORM AND SALVAGE FOR RE-USE/RE-<u>_____</u> INSTALLATION. !----! - EXISTING ELEVATED SLAB TO REMAIN TAKE CARE IN REMOVING PULPIT, LECTERN, CHOIR PEWS, COMMUNION RAIL, CLERGY SEATS, _ _ _ _ _ _ _ _ _ _ _ _ _ _ _ _ ALTAR, ALTAR PLATFORM. SALVAGE AND RETURN L--------------**|----**______ ______ _ _ _ _ _ _ _ _ _ _ -----_______ TAKE CARE IN REMOVING ALL EXSTING PEWS. -----SALVAGE AND RETURN TO OWNER - — — — — — — — ¬ ------------L----_______ ______ ______ ---------L--------L = = = = = = = = = = = _ _ _ _ _ _ _ _ _ _ _ _ _ ______ · |-----REMOVE EXISTING WINDOW. _ _ _ _ _ _ _ _ _ _ ______ REFER TO 7G/A1.10. ______ _____ TAKE CARE IN REMOVING CORK TILE FLOORING FROM NARTHEX AREA DEFINED HERE AND SALVAGE FOR RE-USE/RE-INSTALLATION. AT BOTH RESTROOMS TAKE CARE IN REMOVING THE FOLLOWING AND SALVAGE FOR RE-USE / RE-INSTALLATION: - WOOD SLAT WALL CLADDING - WOOD WALL PANELING - WOOD WALL BASE - CORK TILE FLOORING REMOVE EXISTING FLAT AND SLOPED CEILING.

7A <u>DEMOLITION - LEVEL 1</u> 1/8" = 1'-0"

PROJECT INFORMATION

11

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KEY PLAN

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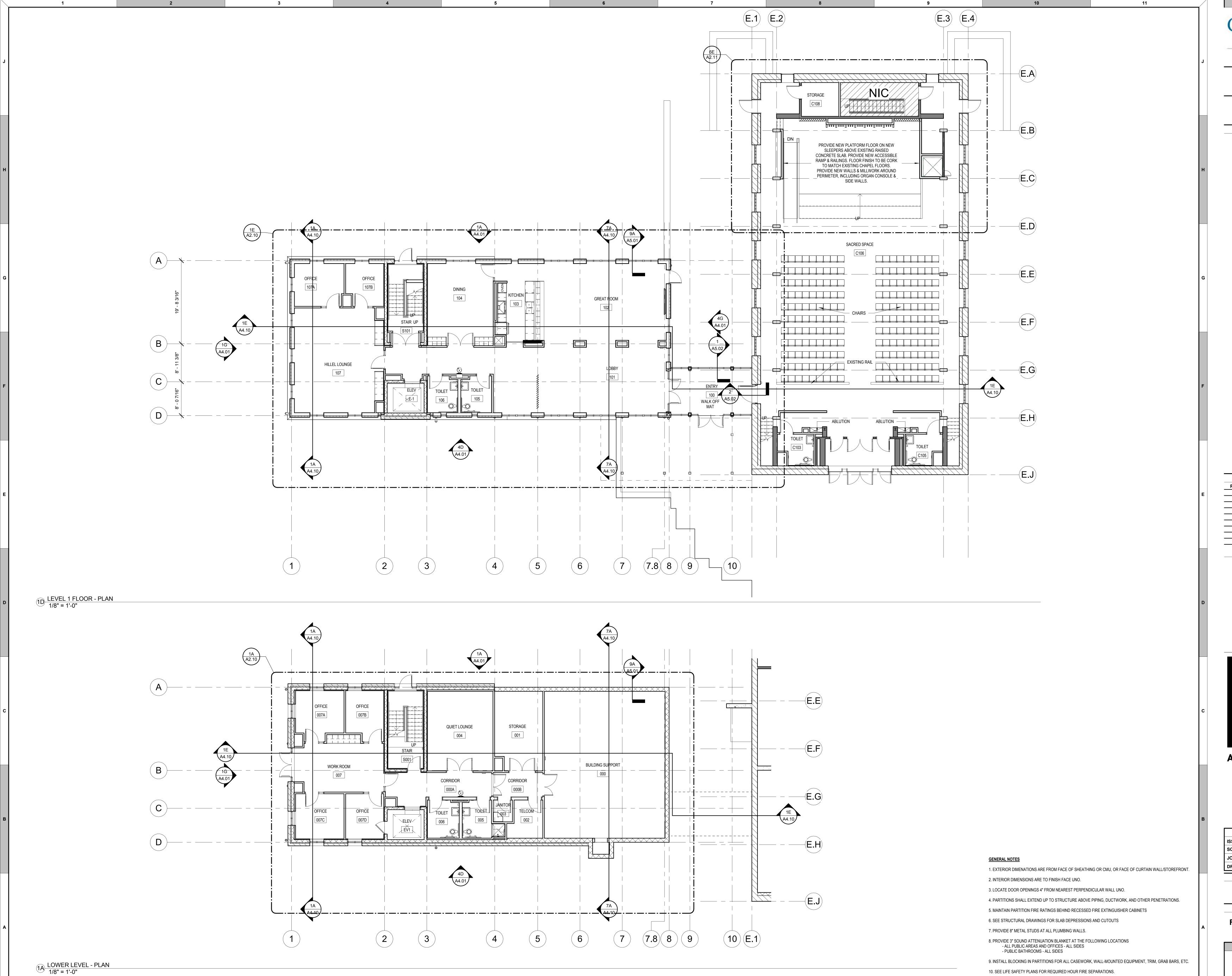
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PLANS DRAWING NUMBER



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PROJECT DESIGN PHASE	

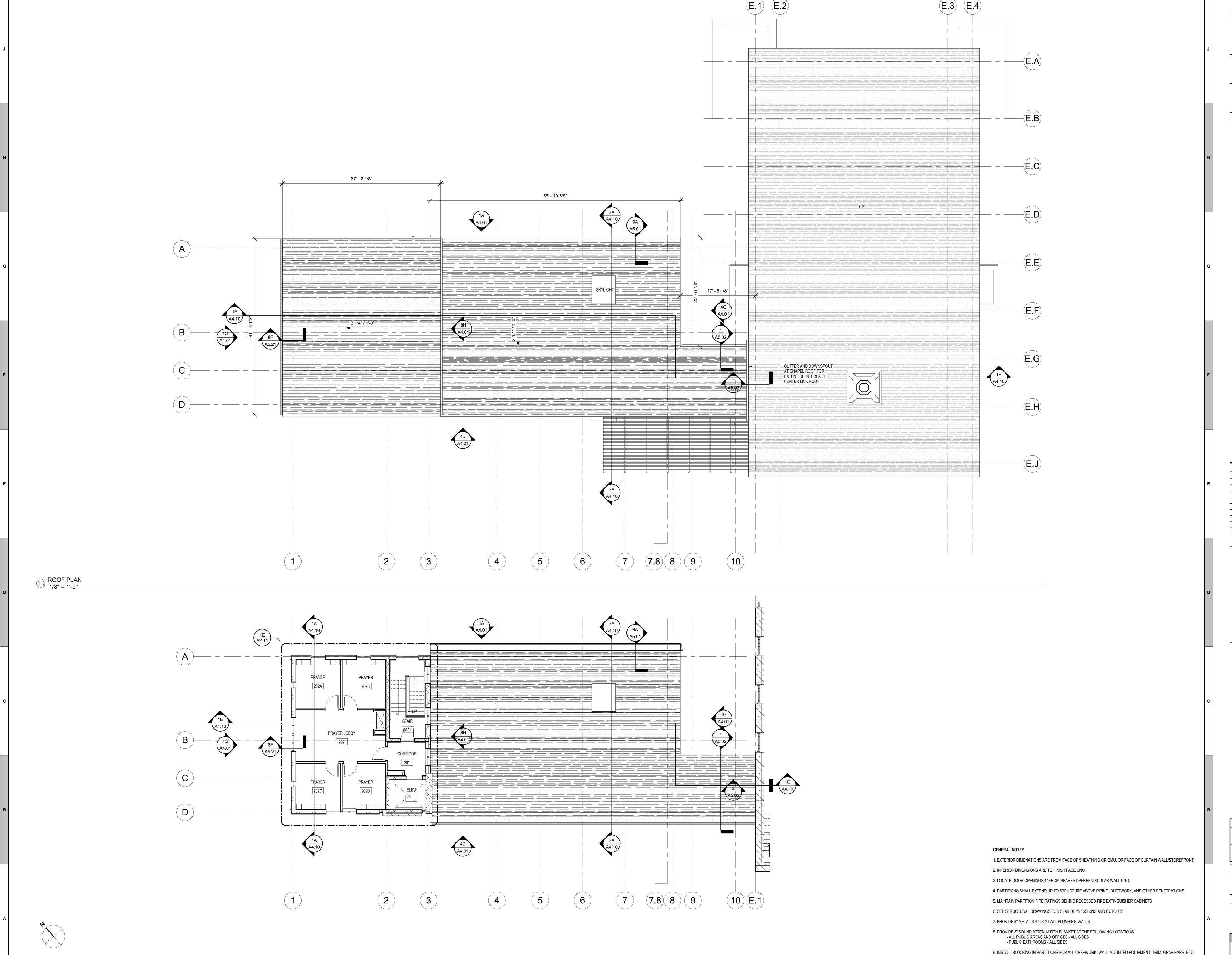
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DOCUMENTS

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FLOOR PLAN - LOWER LEVEL & LEVEL 1

DRAWING NUMBER



1A <u>LEVEL 2 FLOOR - PLAN</u> 1/8" = 1'-0" GOUCHER

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PROJECT DESIGN PHASE	

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DOCUMENTS

DRAWING NAME

FLOOR PLAN - LEVEL 2 &

ROOF

DRAWING NUMBER

10. SEE LIFE SAFETY PLANS FOR REQUIRED HOUR FIRE SEPARATIONS.

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KEY PLAN

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SCALE: JOB NO.:	1/4" = 1'-0" 21641.00
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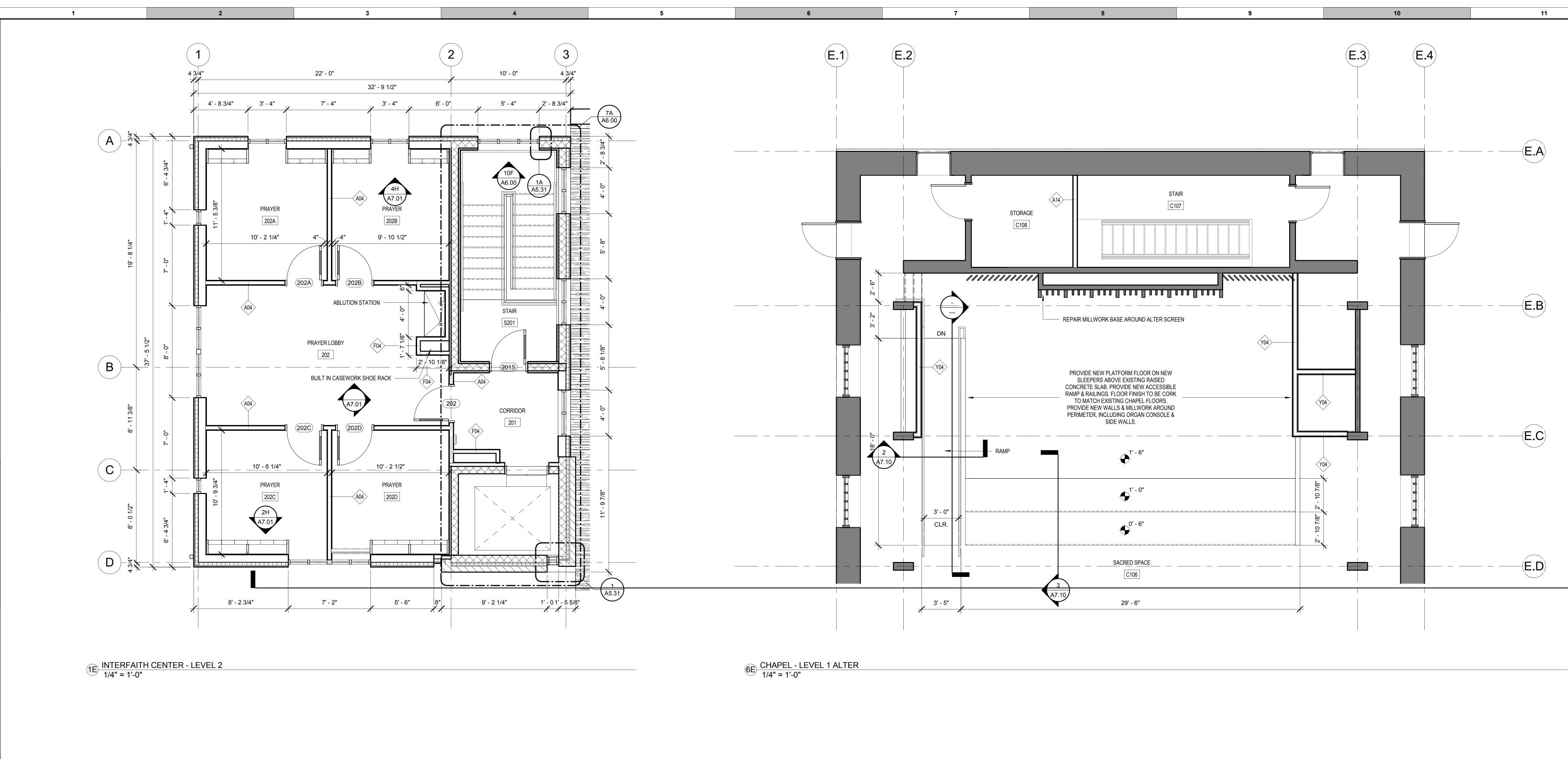
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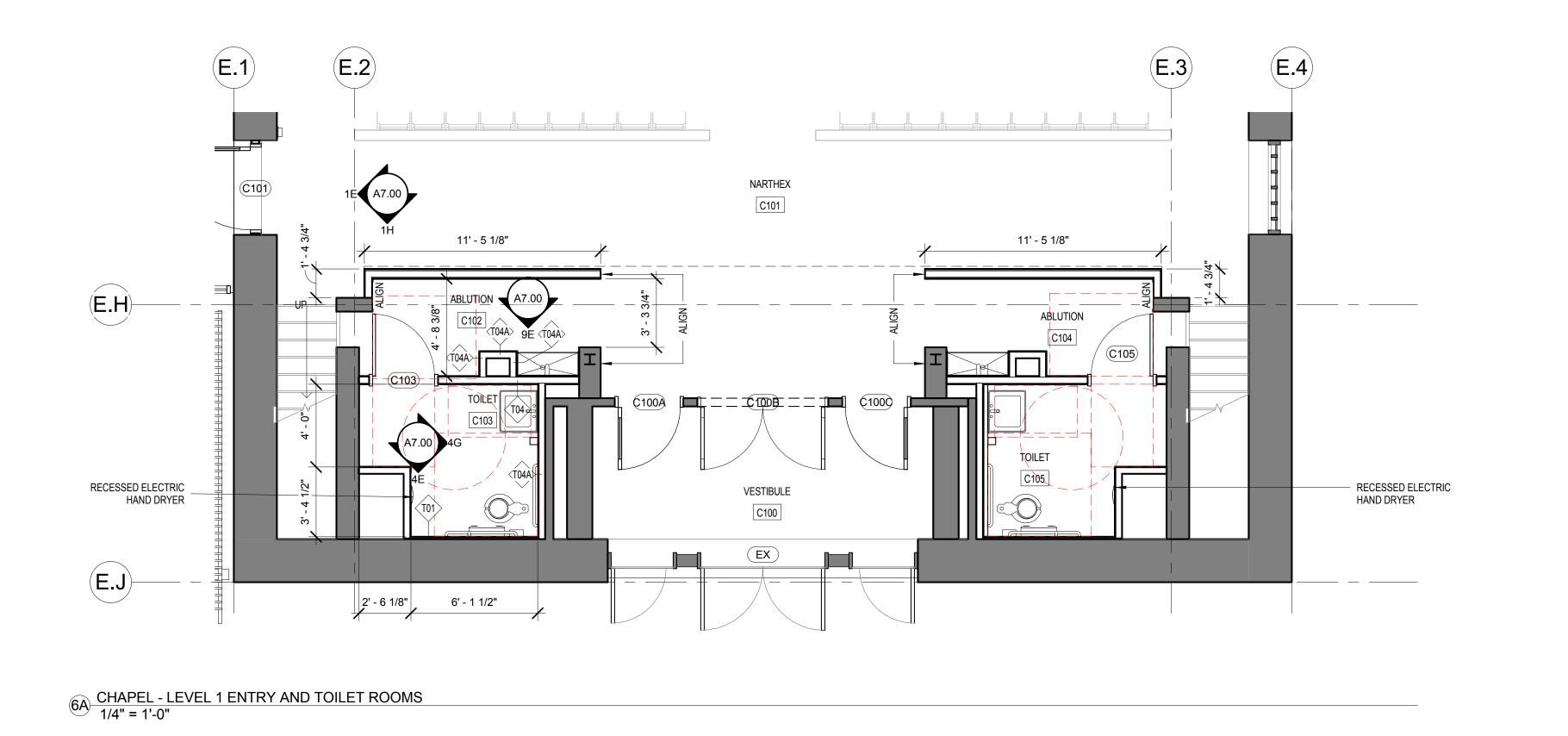
DRAWING NAME

ENLARGED PLANS INTERFAITH CENTER

A2.10

BASEMENT & GROUND LEVEL





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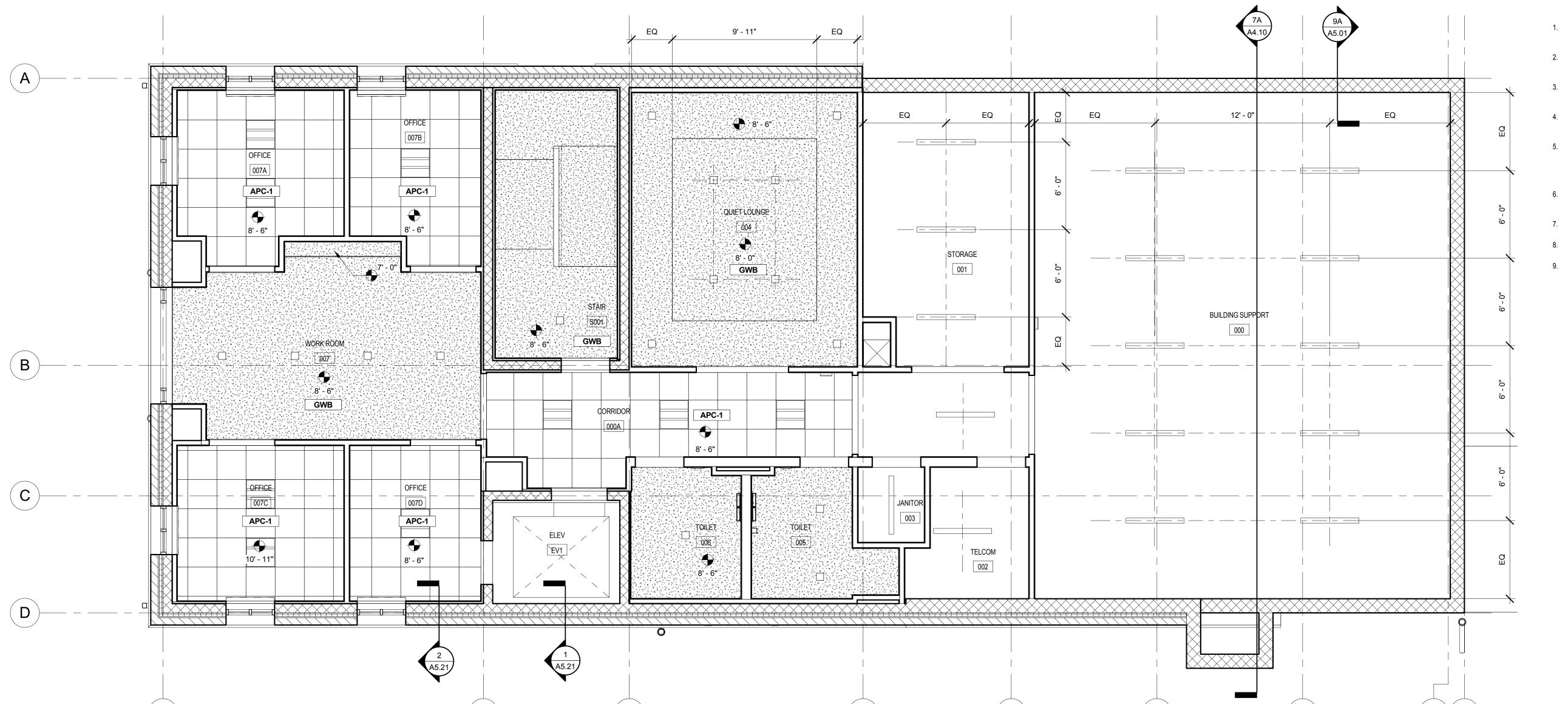
PROJECT DESIGN PHASE **50% CONSTRUCTION**

DOCUMENTS DRAWING NAME **ENLARGED PLANS -**

INTERFAITH CENTER LEVEL 2

& CHAPEL DRAWING NUMBER

A2.11



1E LEVEL 1 - REFLECTED CEILING PLAN
1/4" = 1'-0"

LOWER LEVEL - REFLECTED CEILING PLAN
1/4" = 1'-0"

REFLECTED CEILING NOTES

Borders at lay-in acoustical ceiling panels shall be cut to match factory edge profile. No exposed fasteners shall be permitted including poprivets and tappets.

Height of ceilings shall be measured from top of slab to finish face of GWB or face of ceiling grid as indicated on the Reflected Ceiling Plan,

—(**E.G**)

All light fixtures are to be installed according to the Architectural Reflected Ceiling Plan. Architect to review ceiling layout including bulkheads

and grid prior to installation.

Light fixture types, quantities and locations only are noted on Architectural Reflected Ceiling Plans. Specifications, switching, exit lights,

emergency lighting, life safety equipment, and circuiting are noted on Engineering documents.

Dimensioned light fixtures are from finished face of partitions to centerline of fixture and from centerline of fixture to centerline of fixture. All fixtures shall be installed in center of ceiling tile unless noted otherwise. Any discrepancies with light fixtures, switches, thermostats, or diffusers as to location between architectural and engineering drawings or between the drawings and existing field conditions shall be clarified with the Architect before proceeding with installation.

Existing wood slat ceiling and square recessed light fixtures at underside of choir loft in Chapel to remain. Patch and repair the ceiling where demolition occurred to match existing.

Provide and install manual recessed roller shades at all windows, UON.

Provide and install motorized recessed roller shades at windows in double height Great Room.

Provide and install K-13 Spray On Acoustic Treatment to underside of structural deck thoughout the Lower Level.

LIGHT FIXTURE LEGEND

☐ 6" SQ. RECESSED LED DOWNLIGHT

DECORATIVE LED SCONCE - ALLOW \$300 PER FIXTURE

2X2 RECESSED DIRECT/INDIRECT LED

DECORATIVE VANITY SCONCE

8'-0" UTILITY LINEAR LED PENDANT FIXTURE

EXISTING CHAPEL PENDANT FIXTURE - RELAMP WITH LED

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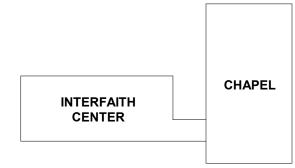
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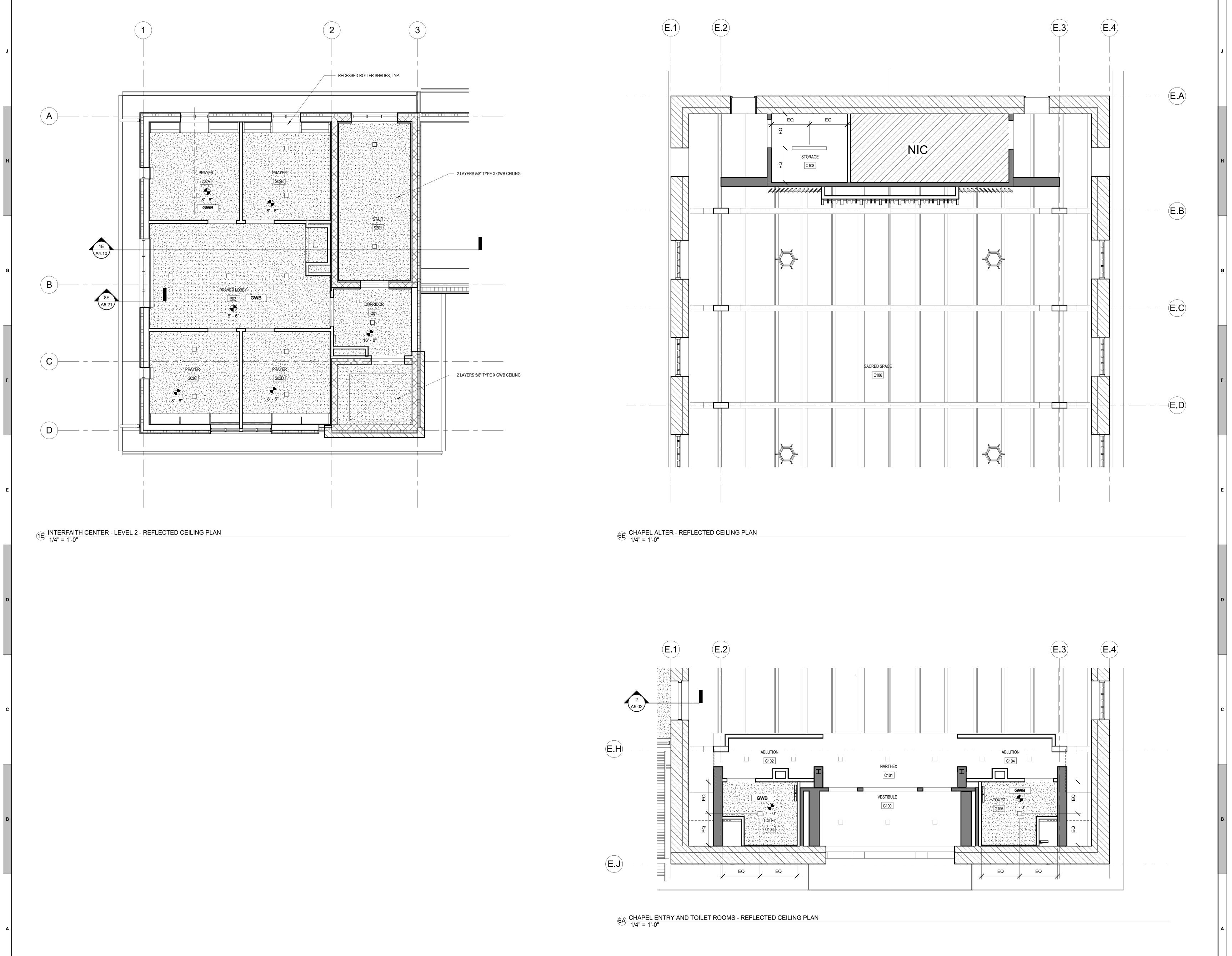
DOCUMENTS

DRAWING NAME

REFLECTED CEILING PLAN -LOWER LEVEL & LEVEL 1

DRAWING NUMBER

A3.10



3

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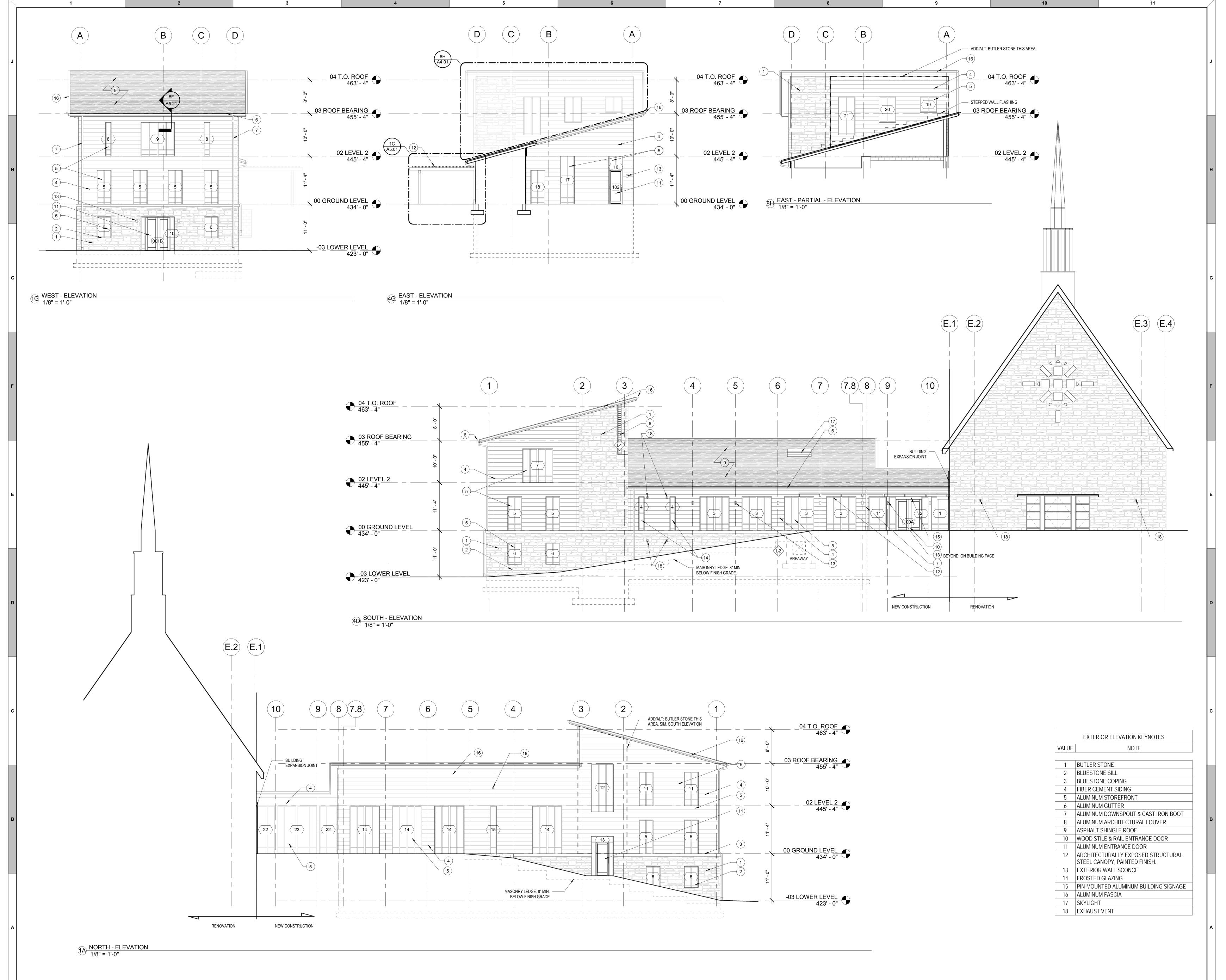
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| DRAWN BY: ASG |
| PROJECT DESIGN PHASE |

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REFLECTED CEILING PLAN -LEVEL 2 & CHAPEL

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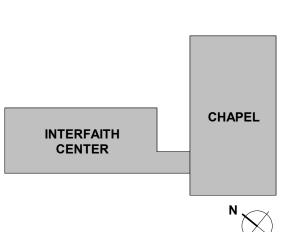
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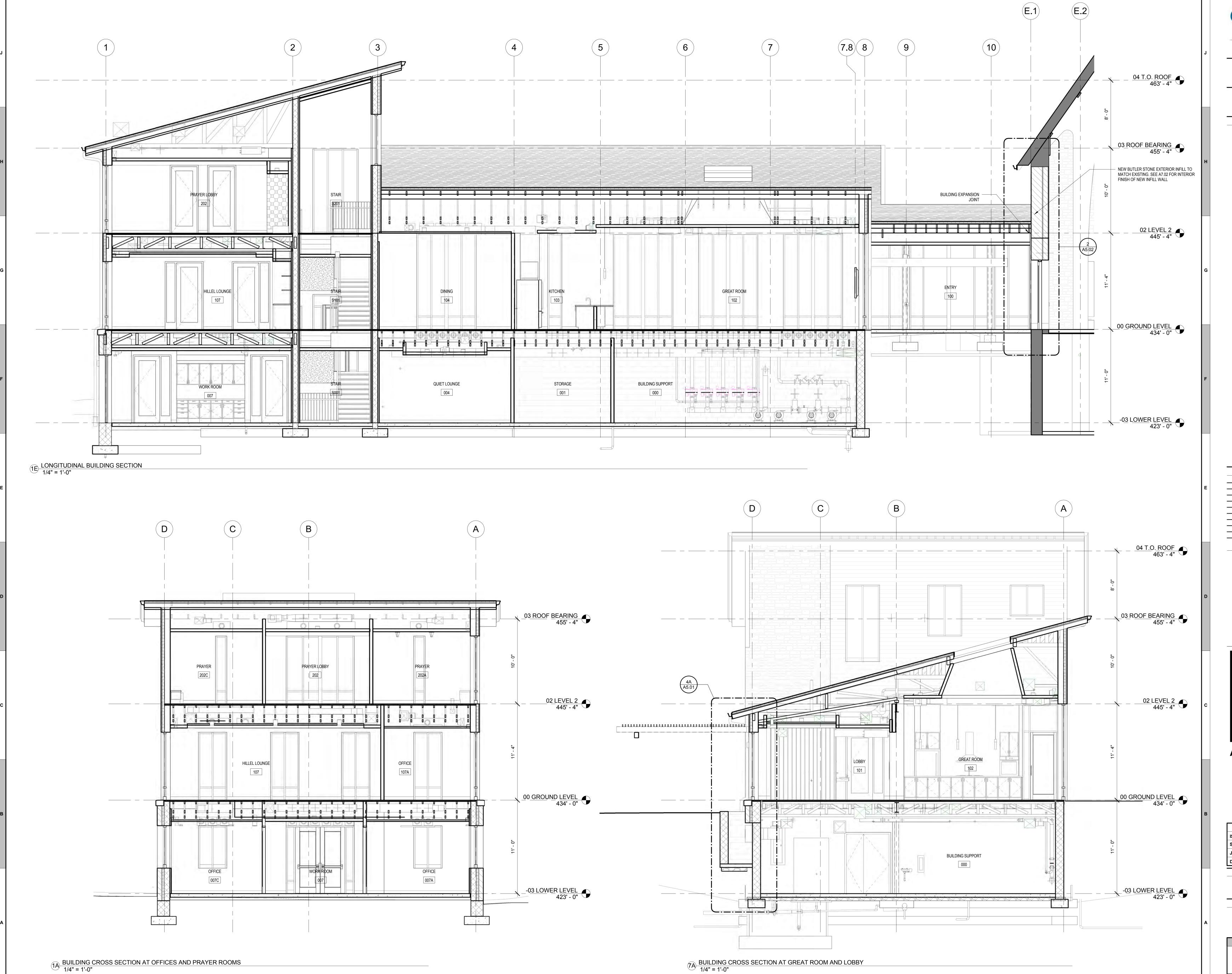
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DRAWING NUMBER



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PROJECT INFORMATION

GOLDSMITH INTERFAITH CENTER

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 DRAWING INFORMATION

 ISSUE DATE:
 04/28/17

 SCALE:
 1/4" = 1'-0"

 JOB NO.:
 21641.00

 DRAWN BY:
 ASG

 PROJECT DESIGN PHASE

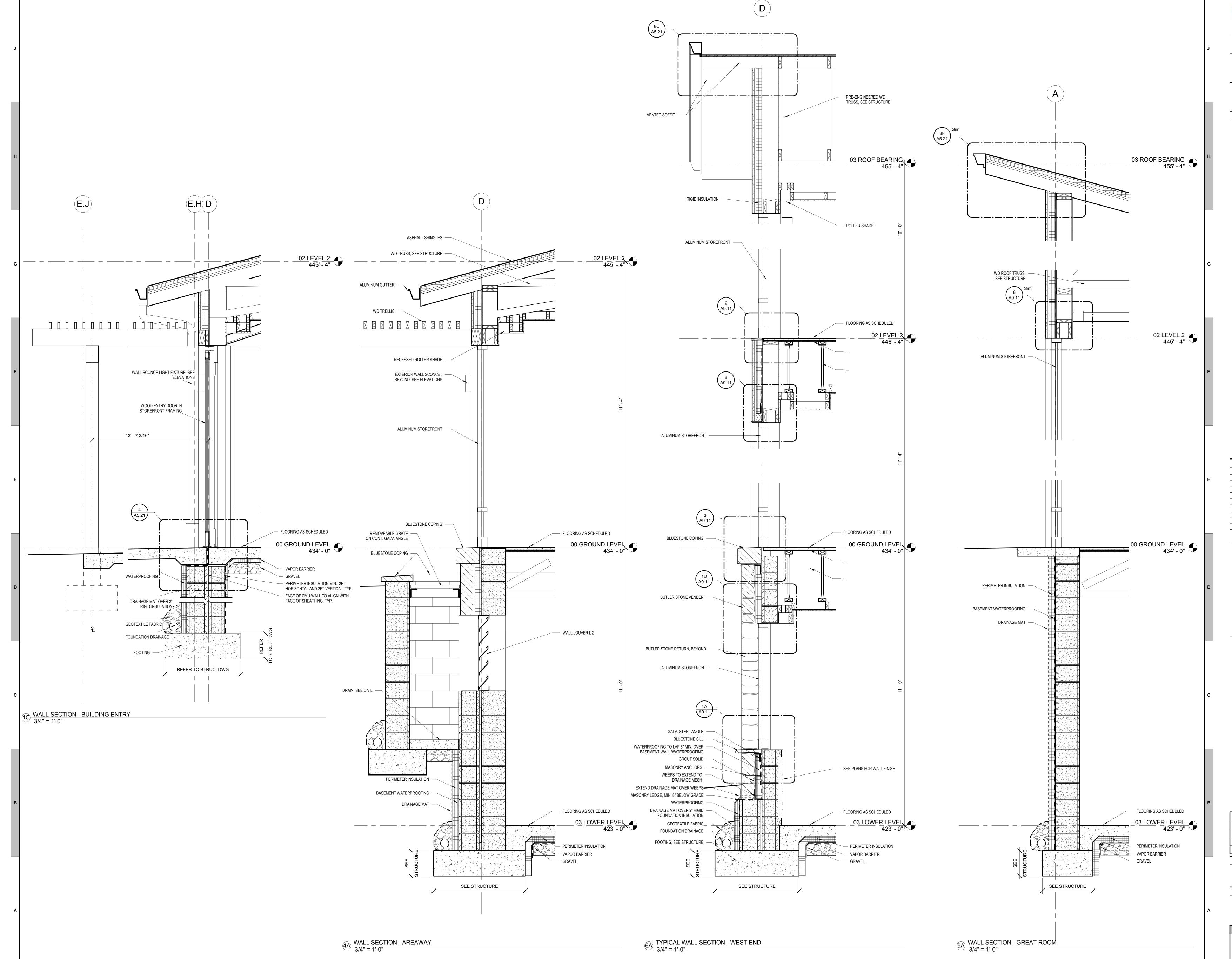
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DRAWING NAME

BUILDING SECTIONS

A4-10



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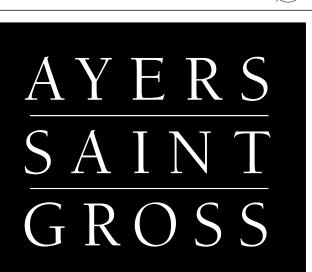
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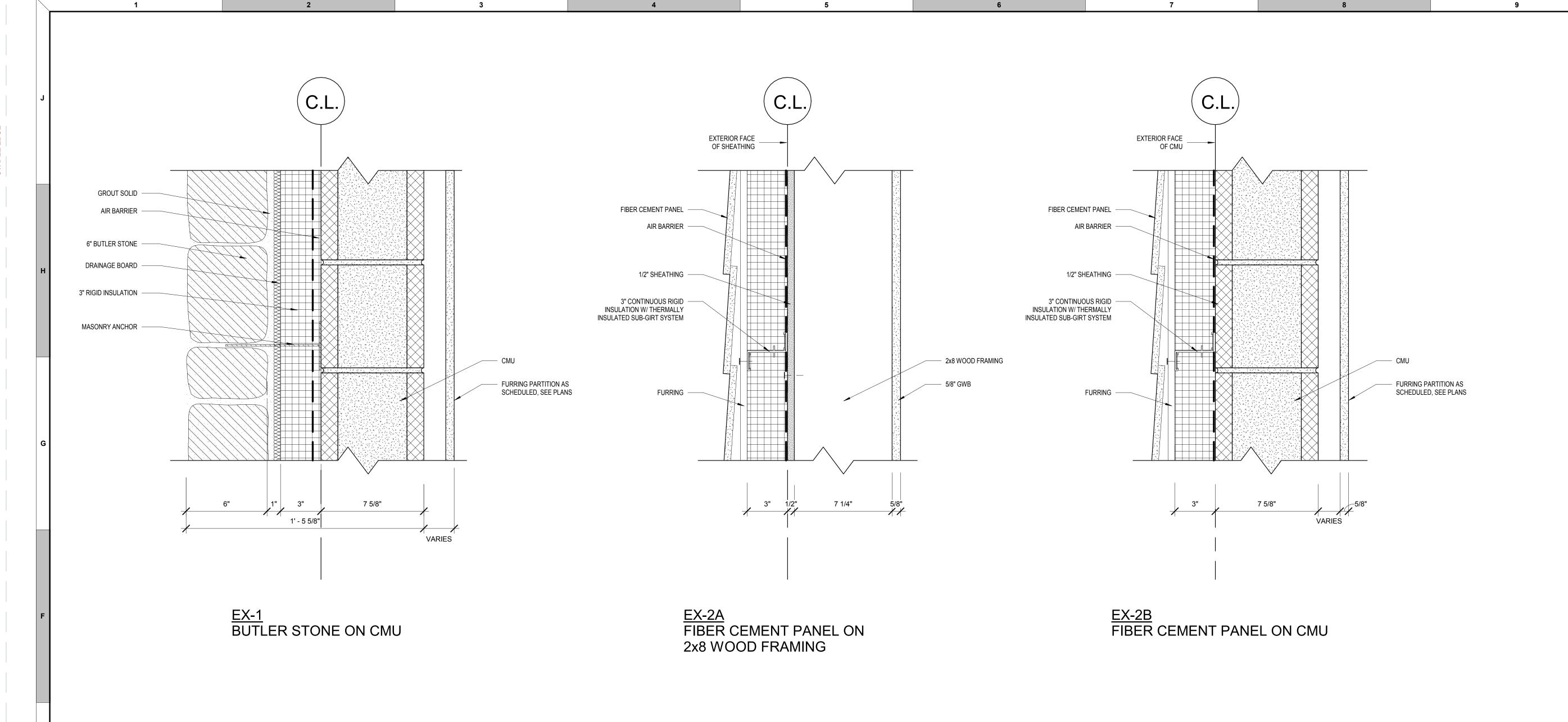
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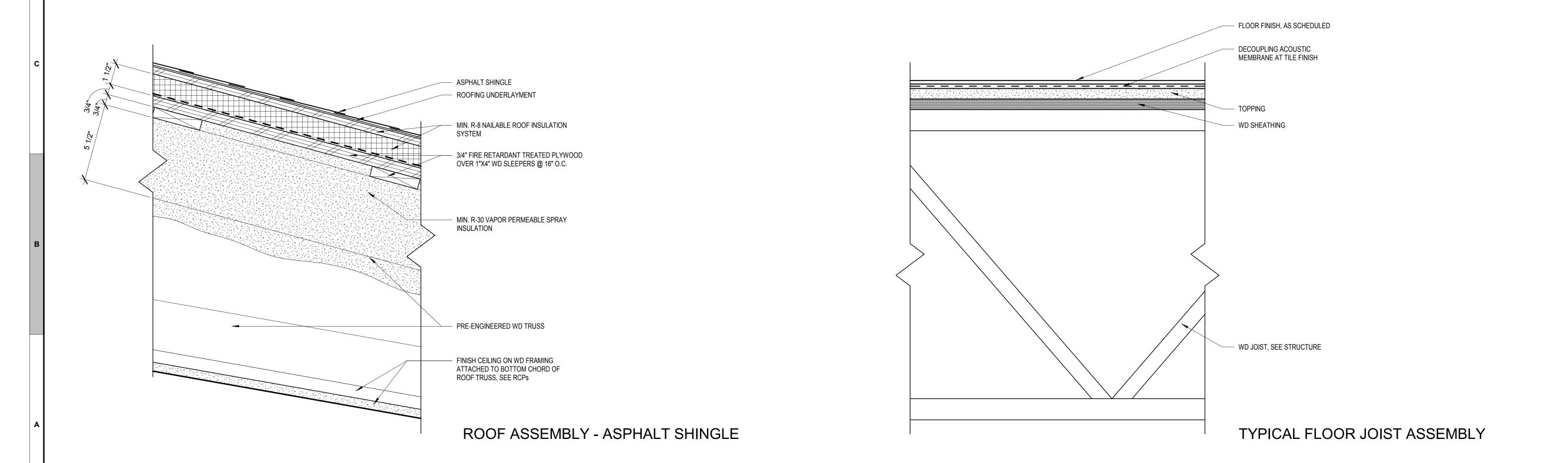
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A5.02





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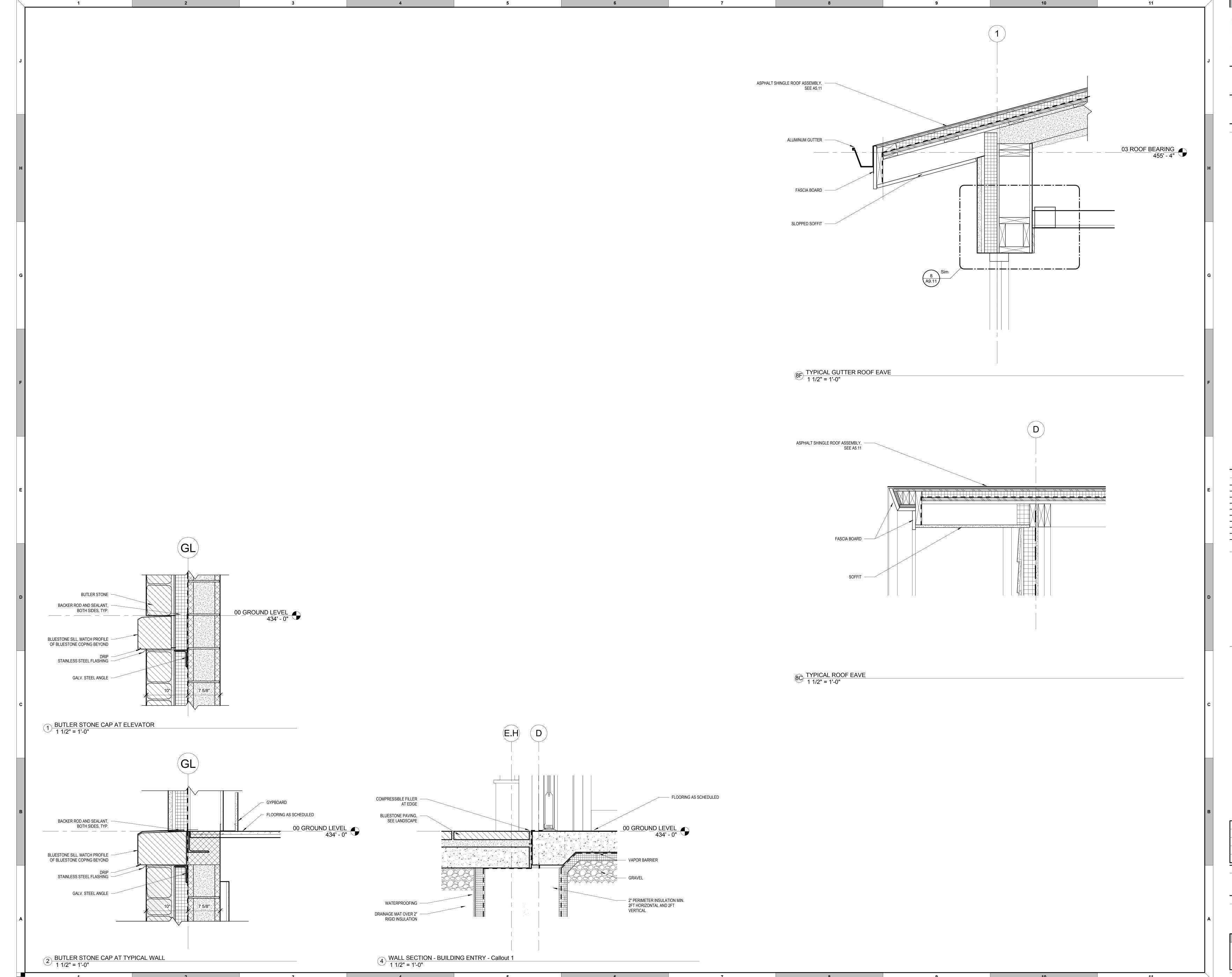
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EXTERIOR ASSEMBLIES

DRAWING NUMBER A5.11



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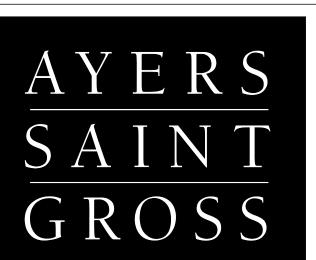
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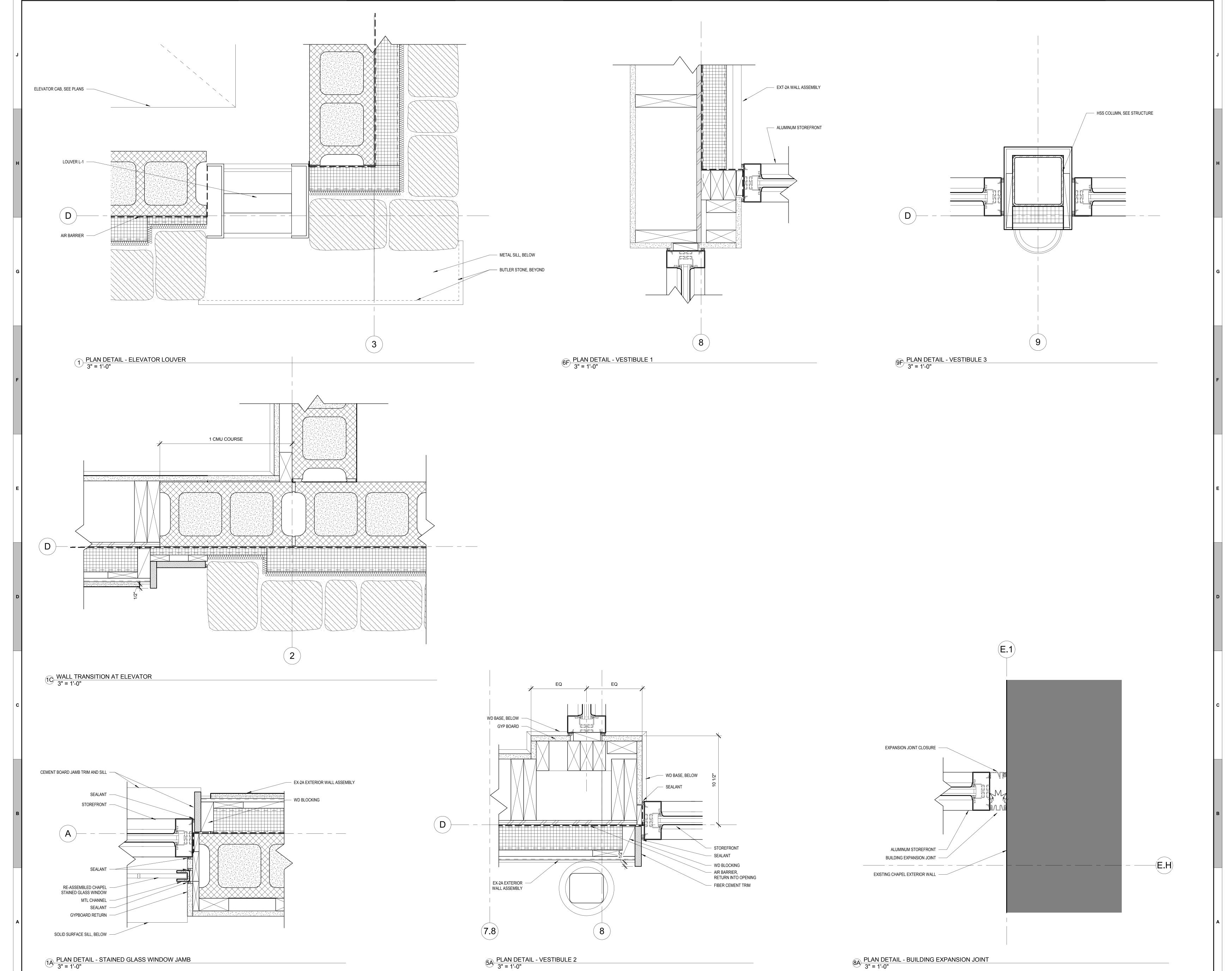
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SECTION DETAILS - EXTERIOR

A5.21



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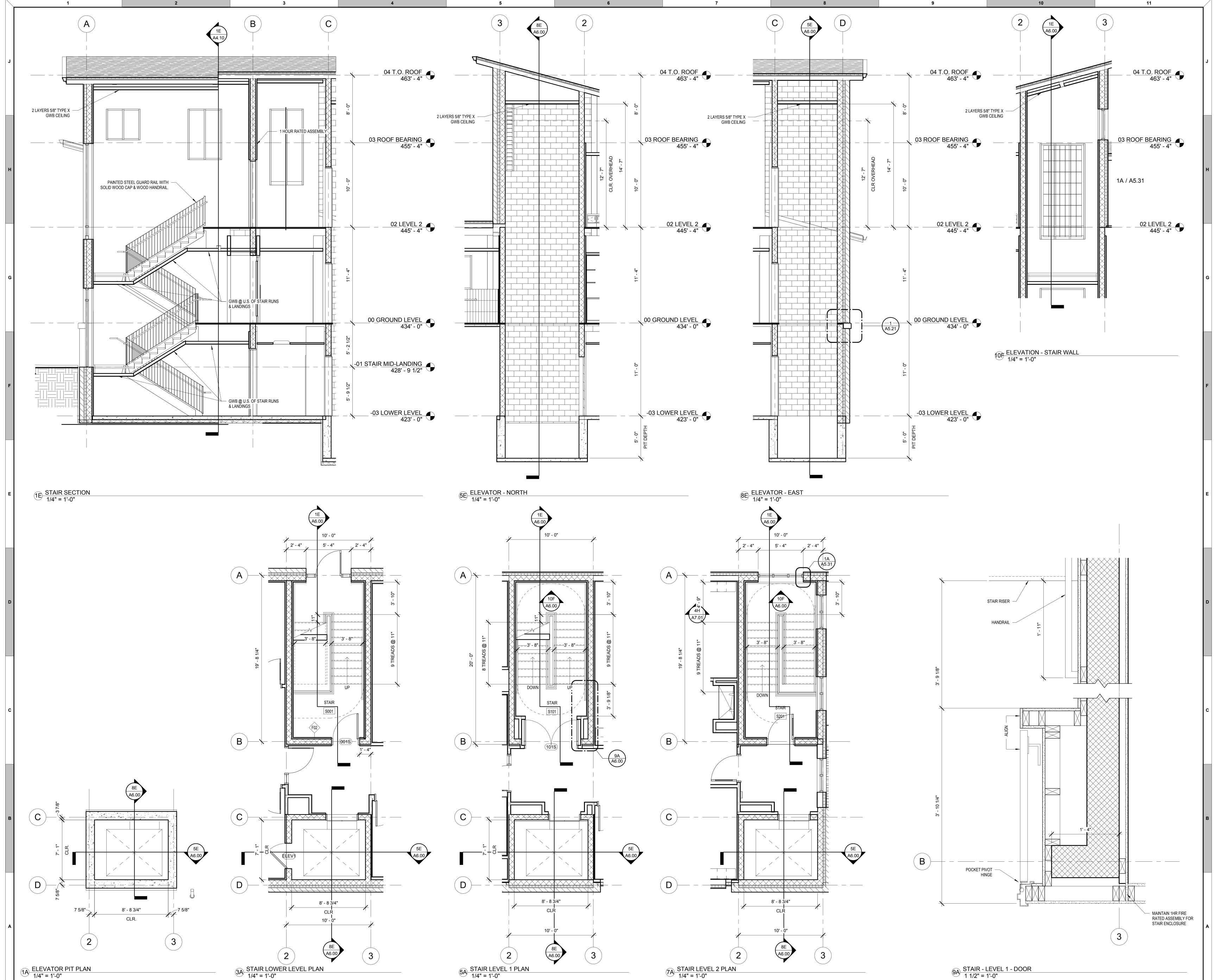
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ISSUE DATE:	04/28/17		
SCALE:	3" = 1'-0"		
JOB NO.:	21641.00		
DRAWN BY: ASG			
PROJECT DESIGN PHASE			
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PLAN DETAILS - EXTERIOR

A5.31



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JOB NO.: 21641.00

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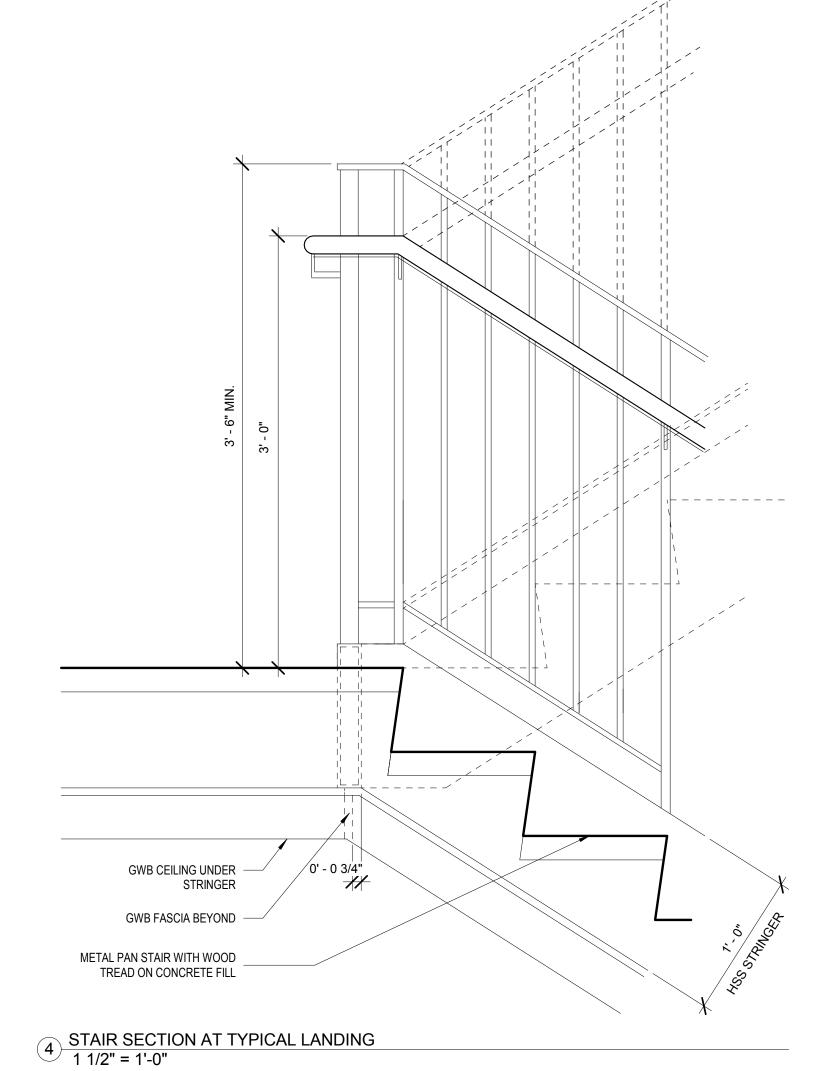
STAIR AND ELEVATOR PLANS & SECTIONS

A6.00

TYPICAL POST MOUNTED HAND RAIL

2" X 12" STRINGER —

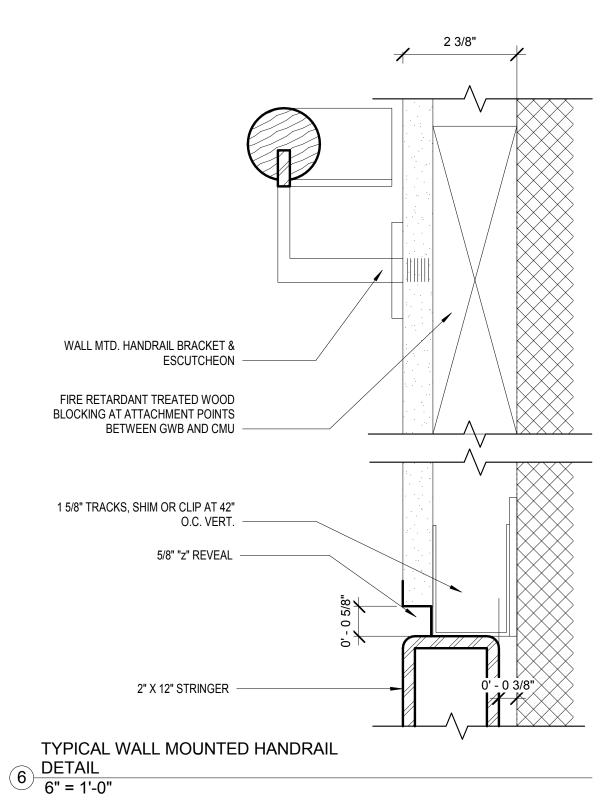
5/8" "z" REVEAL -



<u>|------</u>

GUARD RAIL ELEVATION AT TYPICAL 5 LANDING 1 1/2" = 1'-0"

PAINTED STL INTERMEDIATE POST, PAINTED STL PICKET BOTTOM RAIL HSS STRINGER TOP RAIL ABOVE - PAINTED STL PICKET PAINTED STEEL END POST PROVIDE WELDED CLOSURE AT ENDS OF HSS STRINGER AS REQ'D



REVISIONS REV. # DESCRIPTION **KEY PLAN**

PROJECT INFORMATION

GOLDSMITH INTERFAITH **CENTER**

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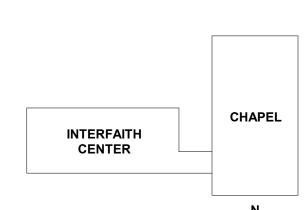
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ISSUE DATE:	04/28/17
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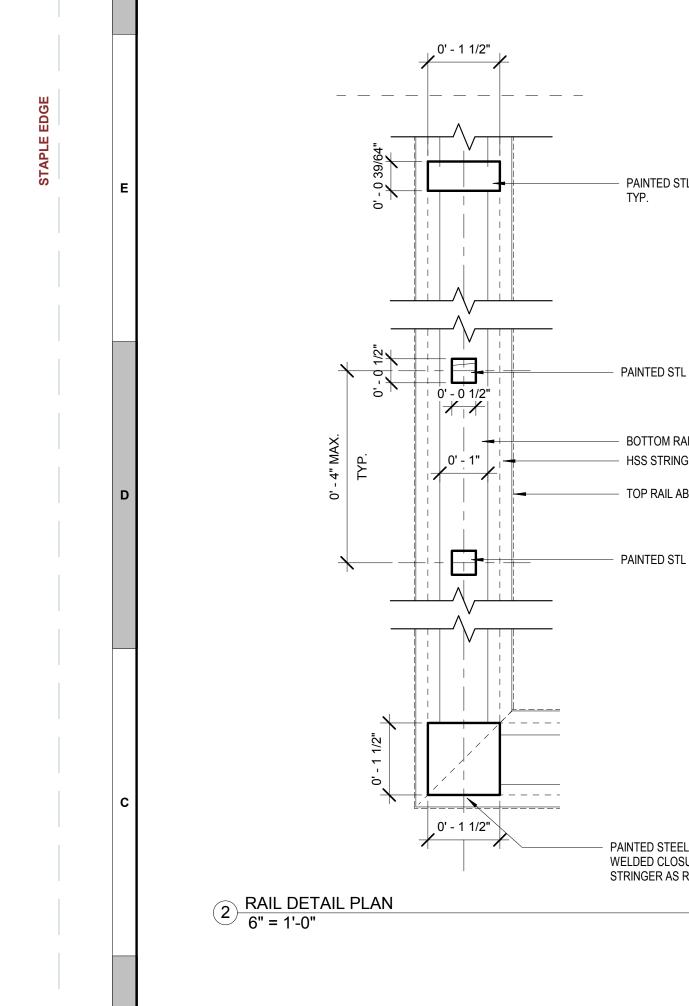
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STAIR & ELEVATOR DETAILS

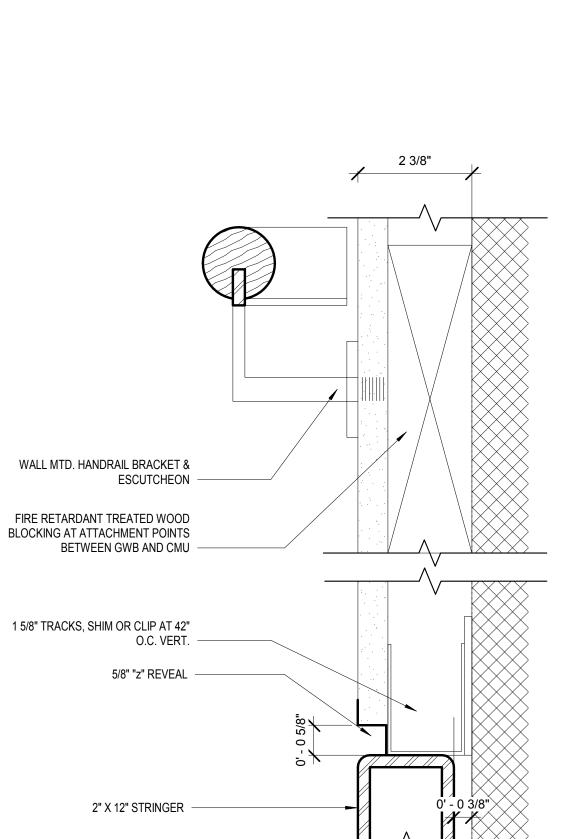
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DRAWING NUMBER A6.20

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1 TYPICAL GUARD RAIL SECTION 6" = 1'-0"



5

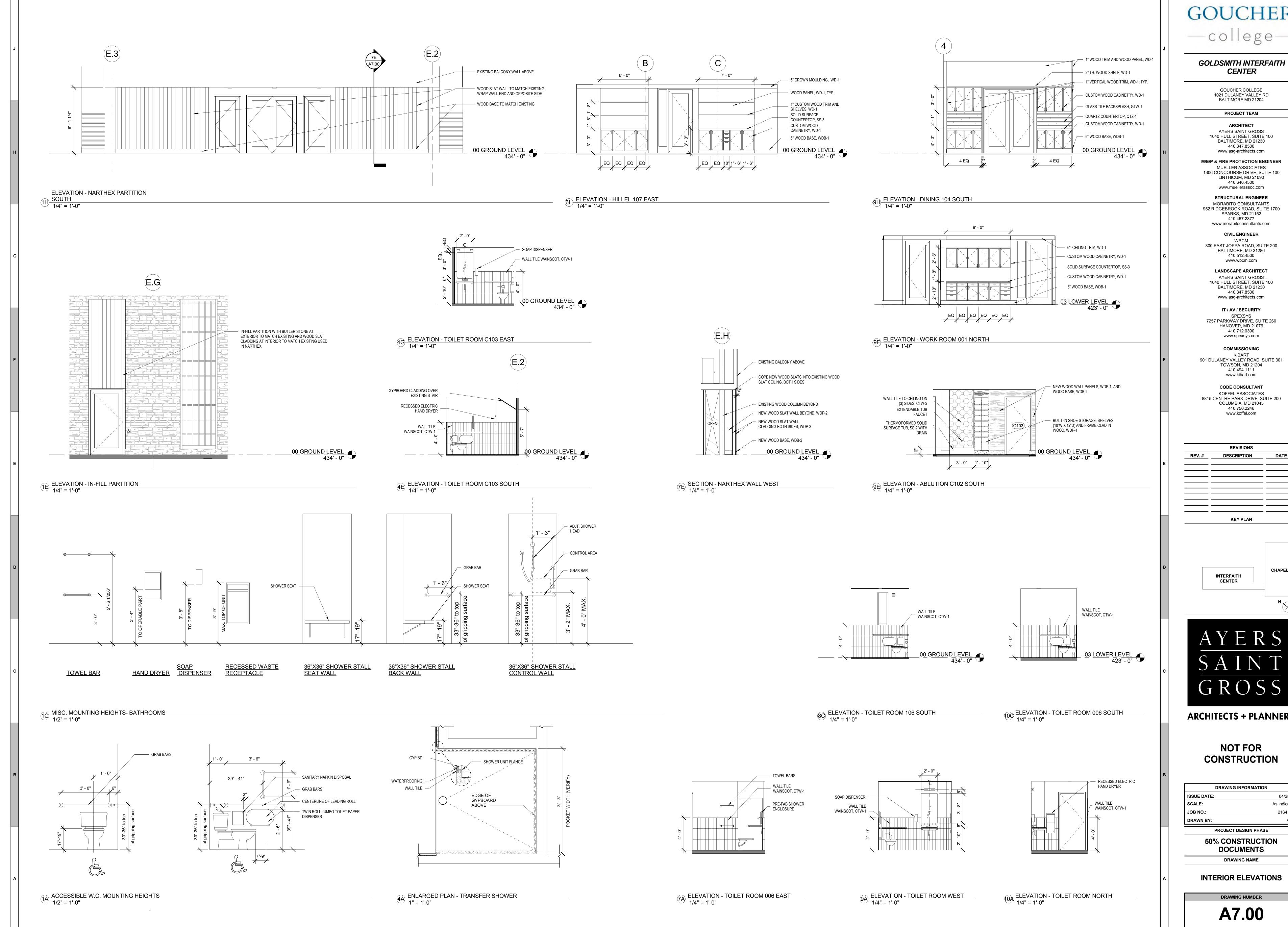
WOOD HAND RAIL CONTINUOUS STEEL BAR

PAINTED STEEL BAR ARM

STIFFENER

0' - 1 123/256" 0' - 1 1/2"

PAINTED STEEL POST



3

PROJECT INFORMATION

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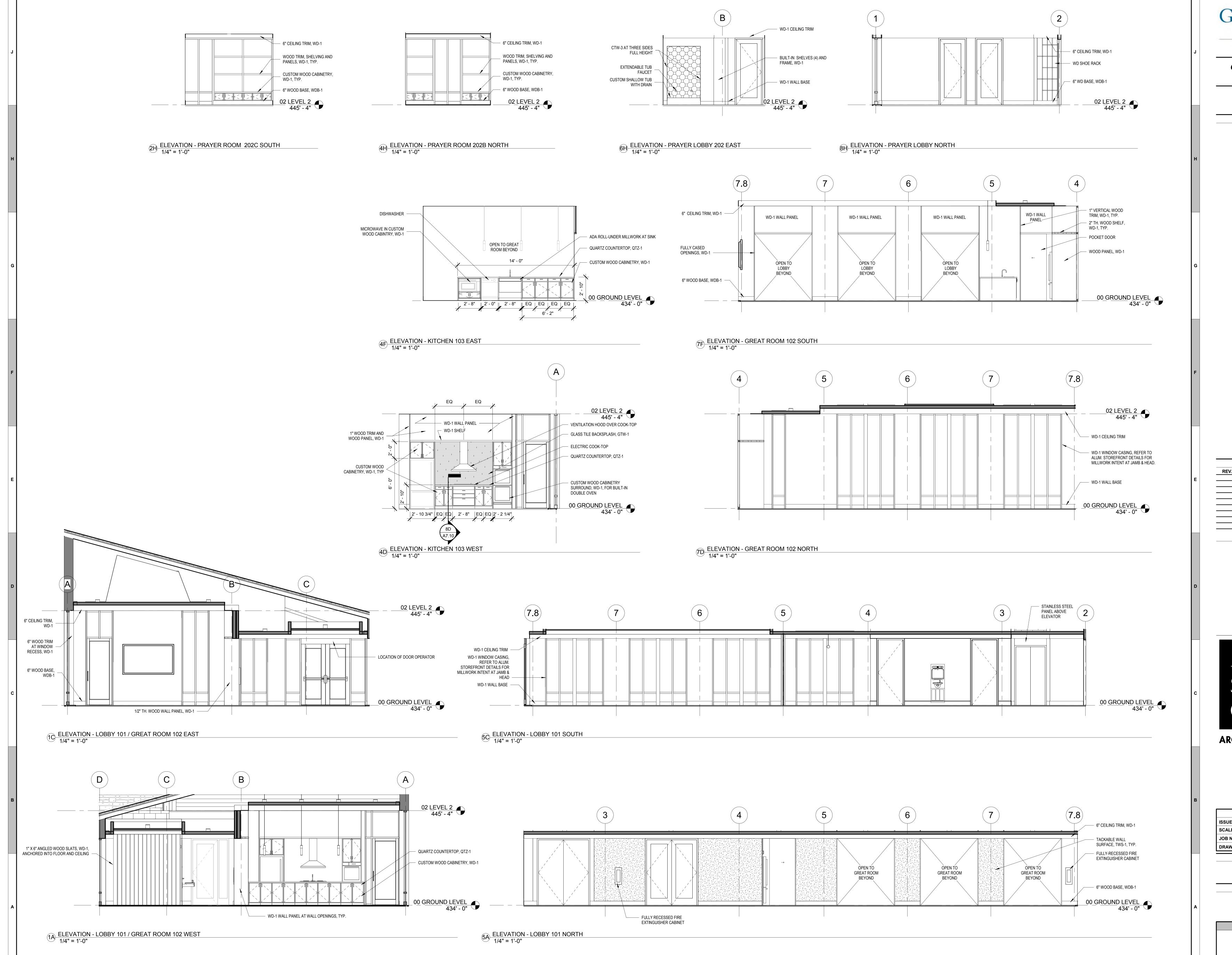
INTERIOR ELEVATIONS

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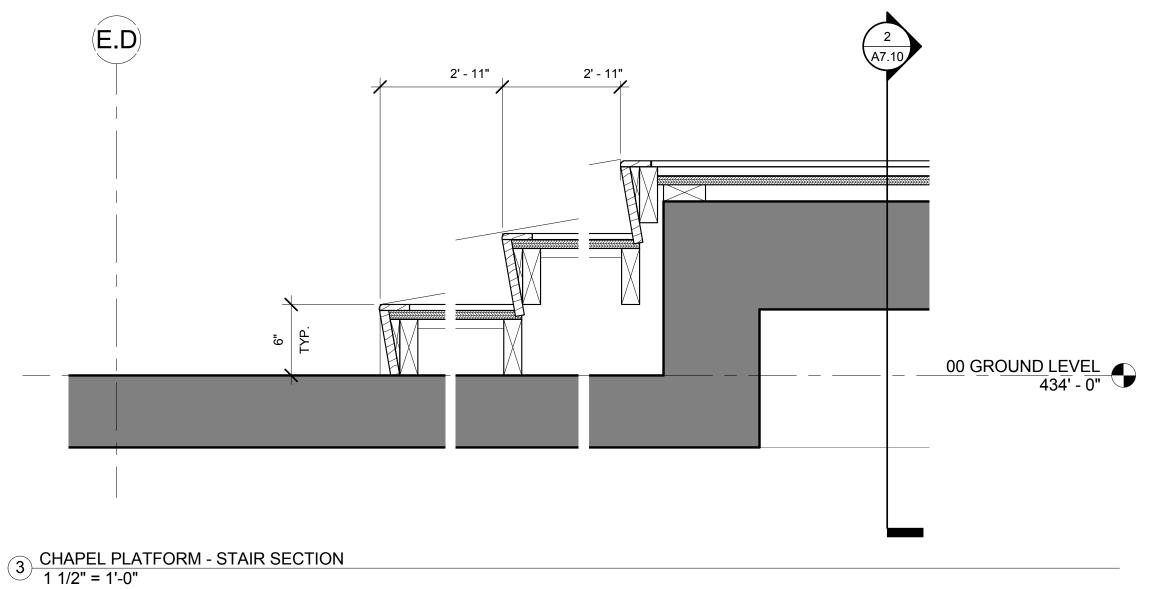
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INTERIOR ELEVATIONS

A7.01



GLASS TILE BACKSPLASH - FULL HEIGHT 2' - 1" 2 CM QUARTZ COUNTERTOP OVER 3/4"
 PLYWOOD SUBSTRATE. EASED AND COUNTER MITERED EDGE - DRAWER PULL 3/4" WOOD PANEL DRAWER - DRAWER GLIDES - CABINET PULL 3/4" WOOD PANEL CABINET DOOR - 3/4" ADJUSTABLE WD SHELF WOOD TOE KICK

PROJECT INFORMATION

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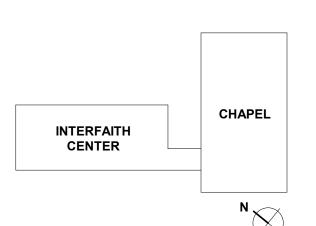
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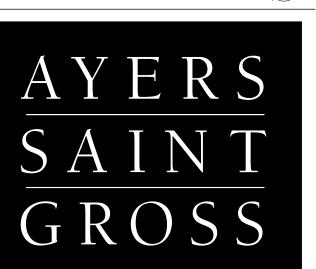
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TOWSON, MD 21204 410.494.1111 www.kibart.com

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REVISIONS REV. # DESCRIPTION **KEY PLAN**





ARCHITECTS + PLANNERS

NOT FOR CONSTRUCTION

ISSUE DATE: 1 1/2" = 1'-0" JOB NO.: DRAWN BY: PROJECT DESIGN PHASE

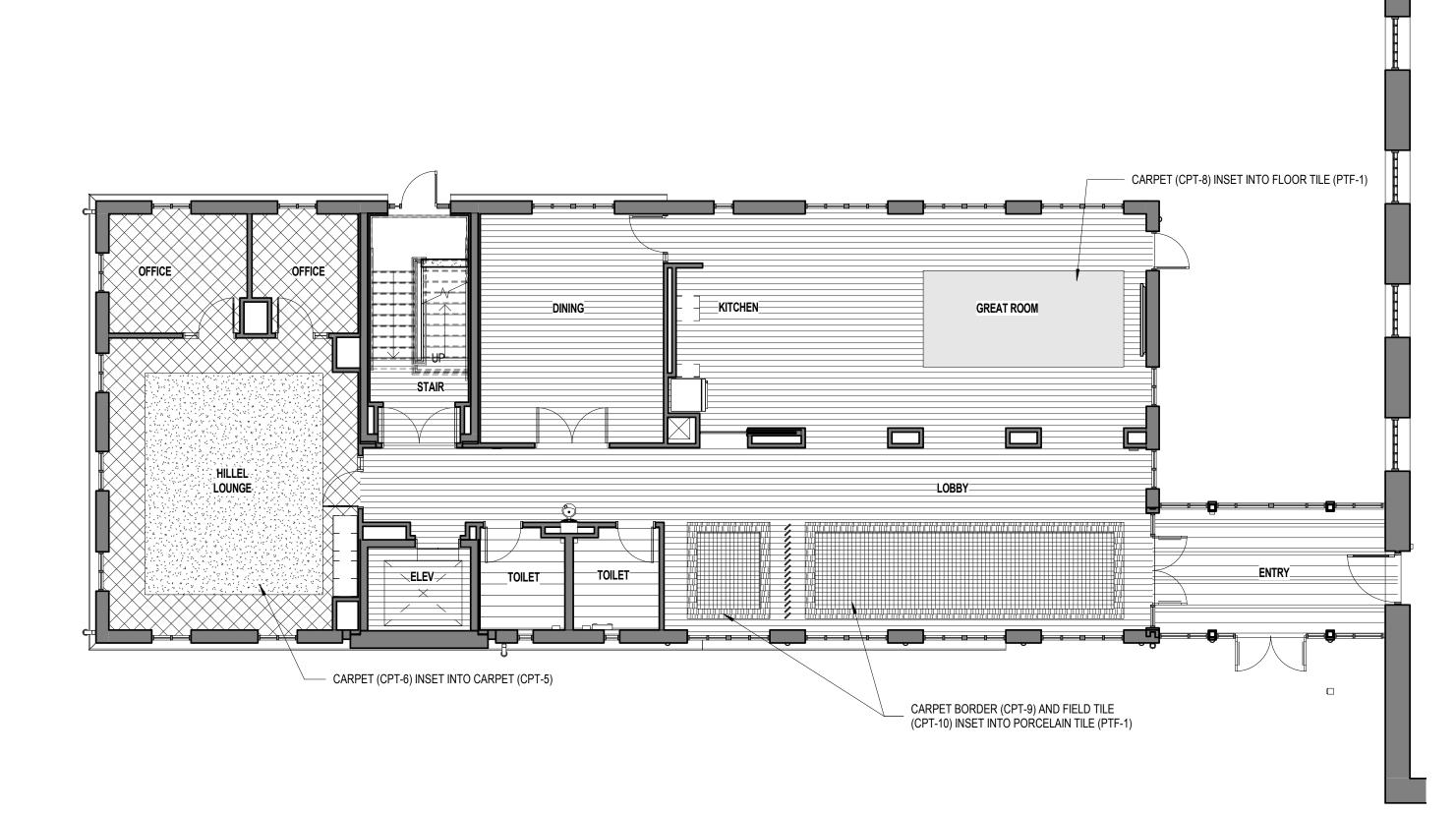
50% CONSTRUCTION

DOCUMENTS DRAWING NAME

INTERIOR DETAILS

DRAWING NUMBER A7.10

1G <u>LEVEL 2 - FINISH PLAN</u> 1/8" = 1'-0"



5

1C <u>LEVEL 1 - FINISH PLAN</u> 1/8" = 1'-0"

 CARPET (CPT-4) INSET INTO CARPET (CPT-3) STORAGE LOUNGE BUILDING SUPPORT WORK ROOM

1A LOWER LEVEL - FINISH PLAN 1/8" = 1'-0"

FLOOR FINISH LEGEND

11

CPT-1 CPT-11 CPT-12 PFT-1 PFT-2

PFT-3

SCO

9

CPT-2 CPT-3 CPT-4 CPT-5 CPT-6 CPT-7 CPT-8 M/E/P & FIRE PROTECTION ENGINEER 1306 CONCOURSE DRIVE, SUITE 100 CPT-9 CPT-10

> STRUCTURAL ENGINEER MORABITO CONSULTANTS 952 RIDGEBROOK ROAD, SUITE 1700 SPARKS, MD 21152 410.467.2377

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PROJECT INFORMATION

GOLDSMITH INTERFAITH

CENTER

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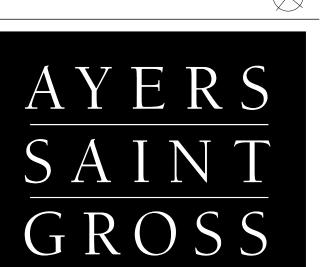
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KEY PLAN

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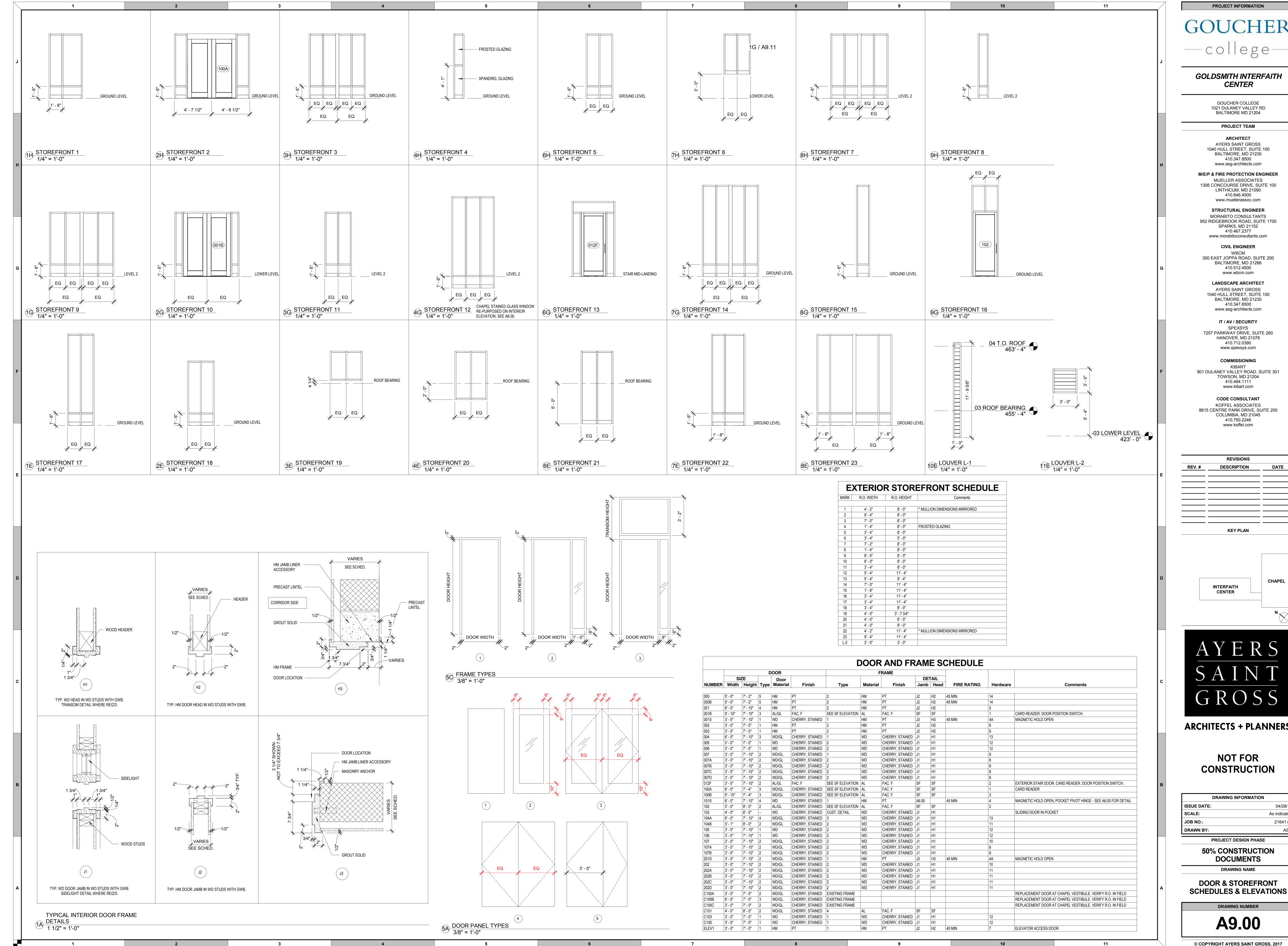
DRAWING INFORMATION	
ISSUE DATE:	04/28/17
SCALE:	1/8" = 1'-0"
JOB NO.:	21641.00
DRAWN BY:	ASG

50% CONSTRUCTION DOCUMENTS

DRAWING NAME

FINISH FLOOR PLANS

DRAWING NUMBER **A8.00**



GOUCHER

PROJECT INFORMATION

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REVISIONS DESCRIPTION

> INTERFAITH CENTER

KEY PLAN

AYERS SAINT GROSS

ARCHITECTS + PLANNERS

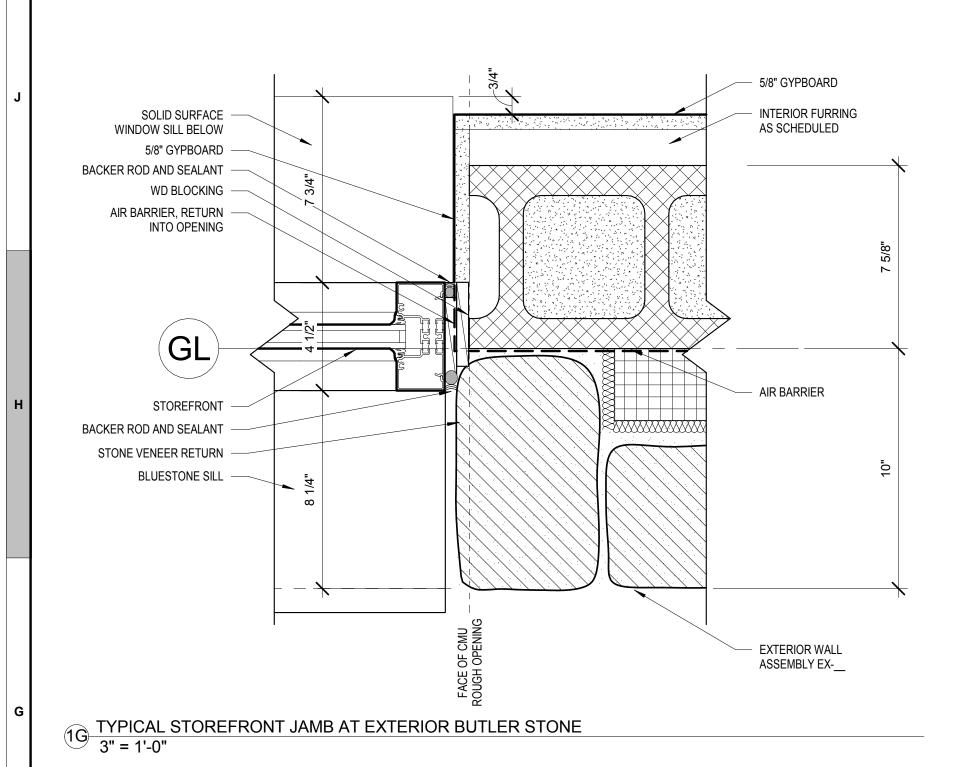
NOT FOR CONSTRUCTION

ISSUE DATE: As indicated JOB NO.: DRAWN BY: PROJECT DESIGN PHASE

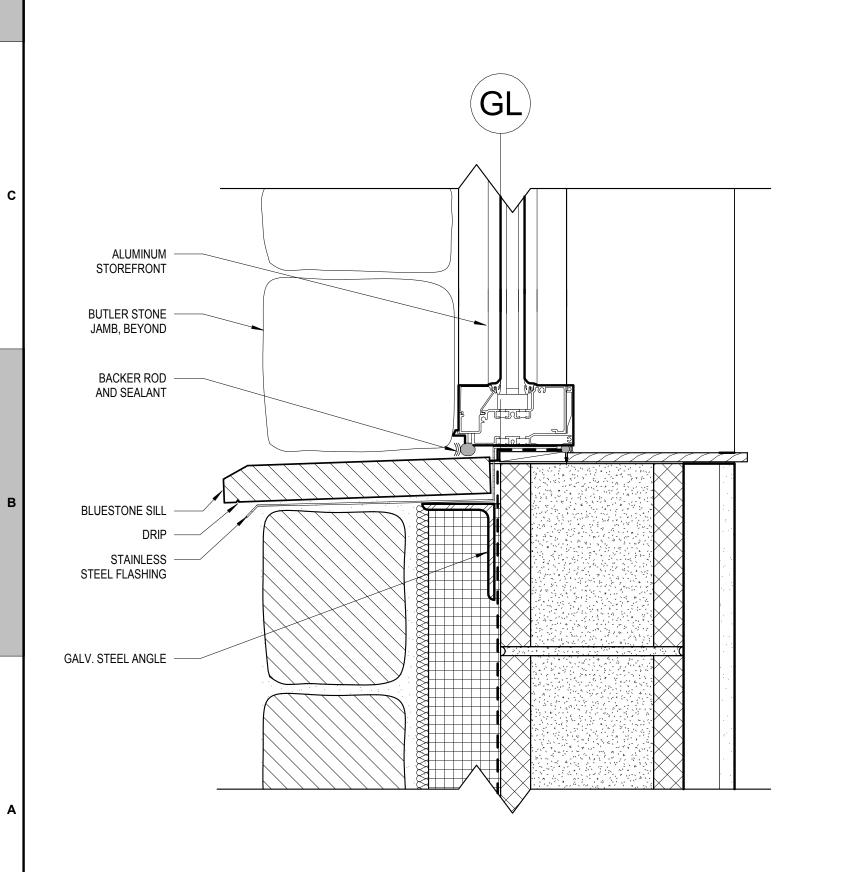
50% CONSTRUCTION DOCUMENTS DRAWING NAME

DOOR & STOREFRONT

DRAWING NUMBER

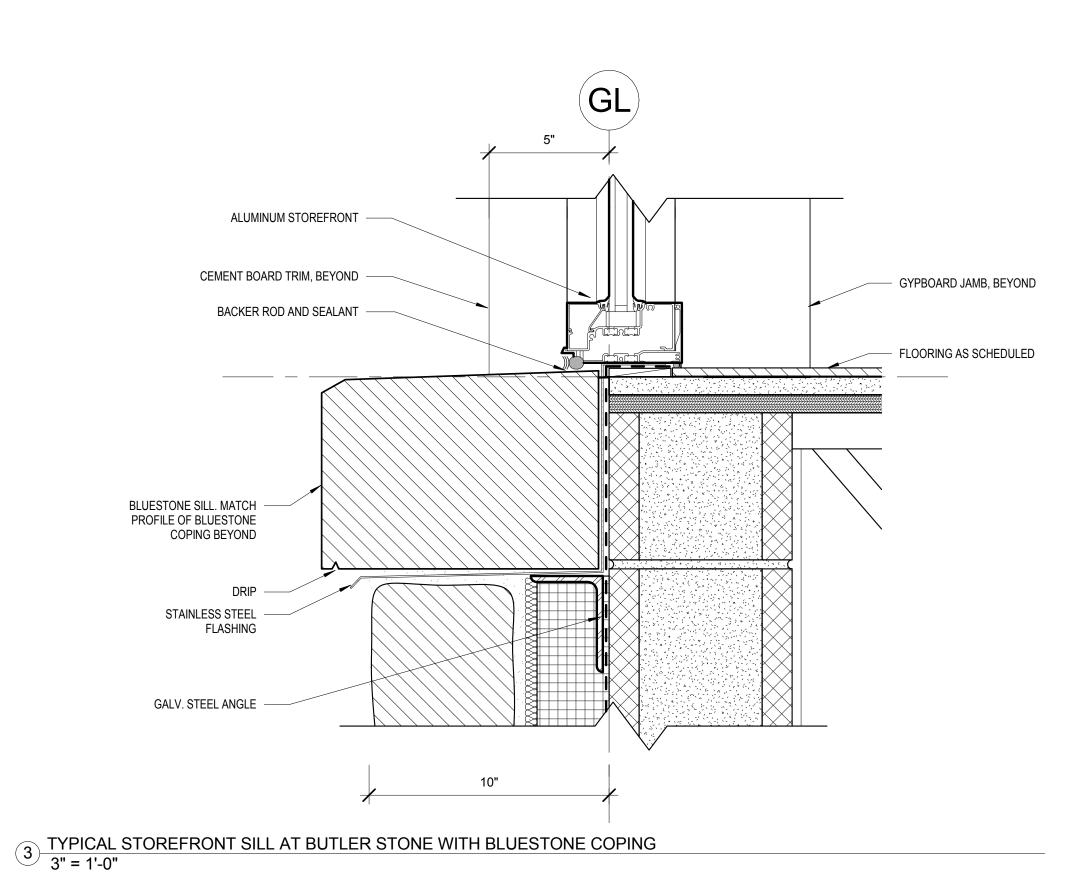


(GL) AIR BARRIER -DRAINAGE BOARD -AIR BARRIER STRIPPING LAP OVER FLASHING STEEL LINTEL, SEE STRUCTURE BACKER ROD AND SEALANT WEEP AT VERTICAL JOINTS -- RECESSED **ROLLER SHADE** STAINLESS STEEL FLASHING GALV. STEEL LINTEL -BACKER ROD AND SEALANT - BACKER ROD AND SEALANT **BUTLER STONE** GYPBOARD RETURN, BEYOND JAMB, BEYOND ALUMINUM STOREFRONT 8 1/4"

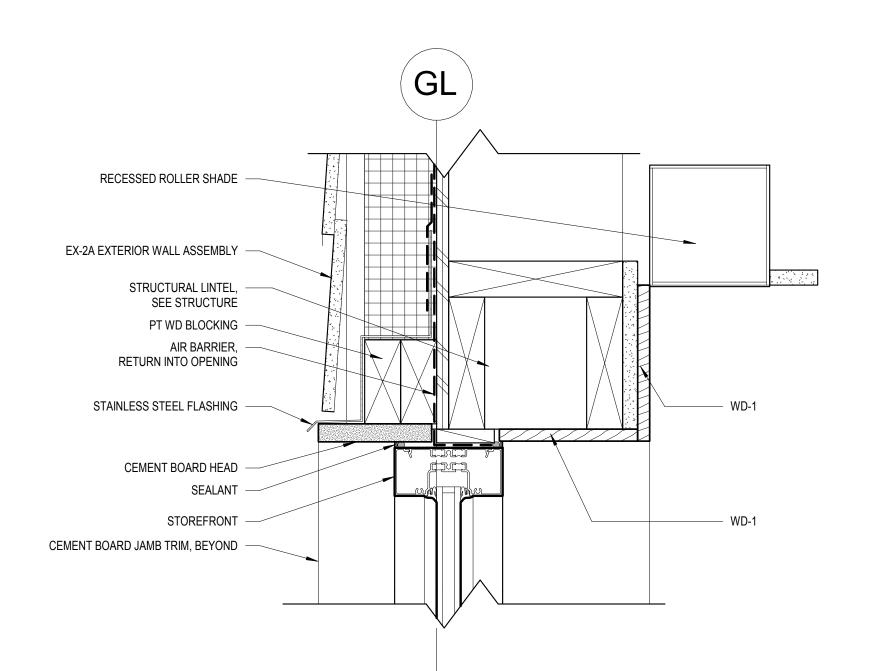


TYPICAL STOREFRONT HEAD AT BUTLER STONE
3" = 1'-0"

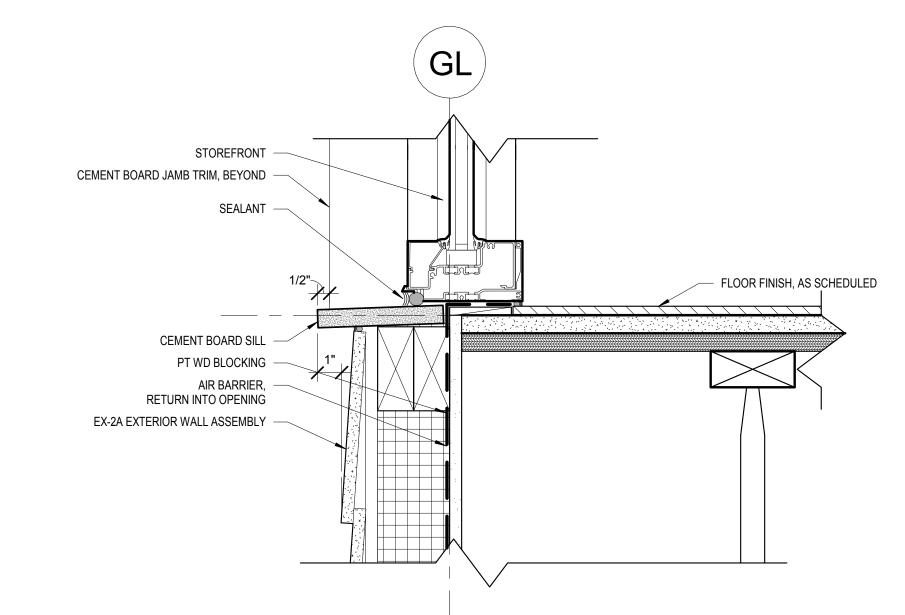
TYPICAL STOREFRONT SILL AT BUTLER STONE
3" = 1'-0"



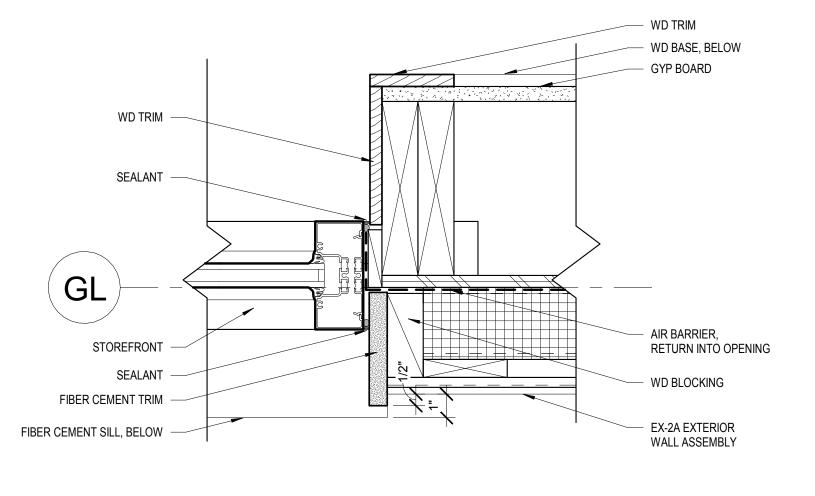
5



8 TYPICAL STOREFRONT HEAD AT CEMENT BOARD SIDING 3" = 1'-0"



2 TYPICAL STOREFRONT SILL AT CEMENT BOARD SIDING 3" = 1'-0"



4 TYPICAL STOREFRONT JAMB AT CEMENT BOARD SIDING 3" = 1'-0"

PROJECT INFORMATION

11

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> GOUCHER COLLEGE 1021 DULANEY VALLEY RD BALTIMORE MD 21204

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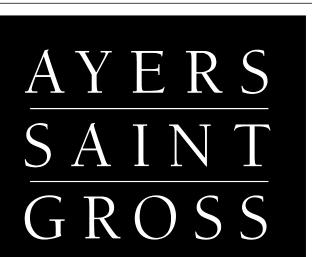
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REVISIONS			
REV.#	DESCRIPTION	DATE	
	KEY PLAN		

INTERFAITH CENTER



ARCHITECTS + PLANNERS

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DRAWING INFORMATION			
ISSUE DATE : 04/28/17			
SCALE:	3" = 1'-0"		
JOB NO.:	21641.00		
DRAWN BY:	ASG		
PROJECT DESIGN PHASE			
50% CONSTRUCTION DOCUMENTS			

STOREFRONT DETAILS

DRAWING NAME

DRAWING NUMBER

NOTES:

1. PARTITIONS ARE DISTINGUISHED ON FLOOR PLANS BY SYMBOL, GRAPHIC DESIGNATION, OR A COMBINATION OF BOTH.

2. THE GRAPHIC SYMBOL DESIGNATION HAS THREE BASE CHARACTERS, AND IN SOME CASES A MODIFIER. PARTITION TYPE LABELS GENERALLY FOLLOW THE FORMAT ILLUSTRATED BELOW, UNLESS OTHERWISE NOTED IN PARTITION SCHEDULE.



A: FIRST CHARACTER INDICATES THE WALL CONSTRUCTION TYPE 0: SECOND CHARACTER INDICATES THE FIRE RATING

4: THIRD CHARACTER DENOTES THE STUD OR CMU THICKNESS (NOMINAL) A: FOURTH CHARACTER (OPTIONAL) IS A MODIFIER; REFERENCED IN THE "OTHER CRITERIA" COLUMN OF THE CORRESPONDING PARTITION SCHEDULE

3

WALL CONSTRUCTION TYPE:

A: STUD/DRYWALL PARTITION C: CMU PARTITION F: FURRING PARTITION

S: SHAFTWALL T: MOISTURE-RESISTANT PARTITION (TLT PARTITIONS) Y: CUSTOM PARTITION

FIRE RATING:

0: NOT RATED .5: 1/2 HR. RATED 1: 1 HR. RATED 2: 2 HR. RATED

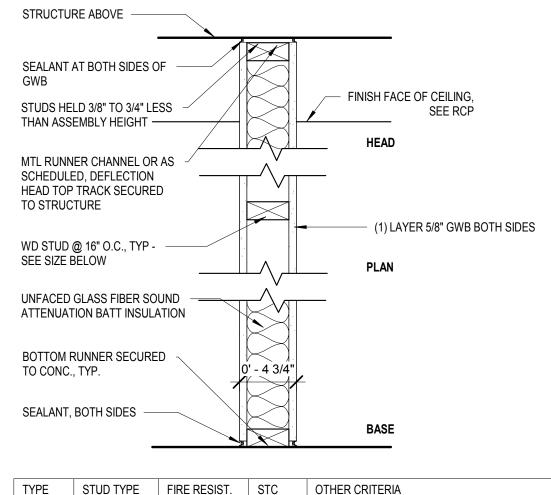
STUD/CMU THICKNESS:

1: 1x WOOD FURRING

2: 1 1/2" WOOD STUD 4: 3 1/2" WOOD STUD OR 4" CMU (NOM.) OR 4" C-H

8: 8" CMU (NOM.)

6: 5 1/2" WOOD STUD



TYPE	STUD TYPE	FIRE RESIST. TEST	STC RATING	OTHER CRITERIA
A04	WD 2"x4"			3-1/2" UNFACED GLASS FIBER SOUND ATTENUATION BATT INSULATION
A06	WD 2"X6"			5-1/2" UNFACED GLASS FIBER SOUND ATTENUATION BATT INSULATION
A08	WD 2"X8"			7-1/2" UNFACED GLASS FIBER SOUND ATTENUATION BATT INSULATION

HEAD

STUD TYPE FIRE RESIST. STC OTHER CRITERIA

TEST

U905

U905

6D CX8: MASONRY PARTITION 1 1/2" = 1'-0"

----- 8" CMU, BEGIN COURSING @ FINISH FLR ELEVATION

1 HR. RATED CMU WALL

2 HR. RATED CMU WALL

6G A0X - WD STUD, DRYWALL PARTITION 1 1/2" = 1'-0"

STRUCTURE ABOVE

CONT. SEALANT, BOTH SIDES

CLIP ANGLE @ 6' O.C., EA SIDE

CONT HORZ REINF.

SEE STRUCTURAL DWGS

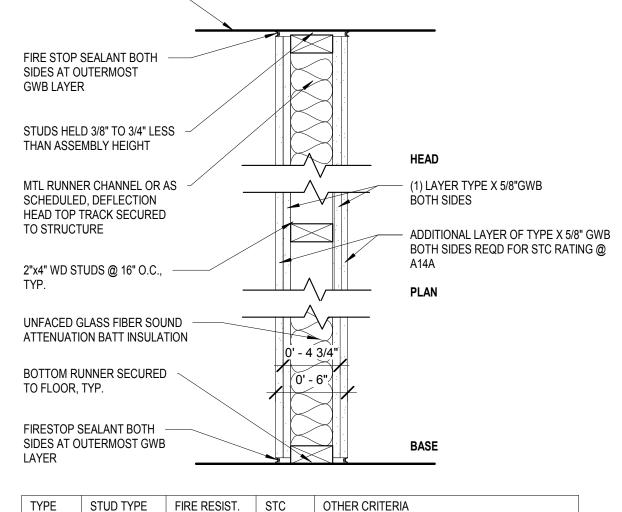
GROUT REINF. CELLS FULL

FOR VERT REINF.

C18 8" CMU

C28 8" CMU

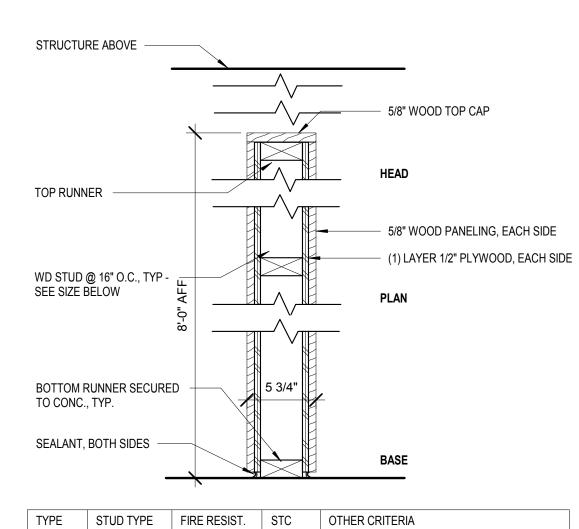
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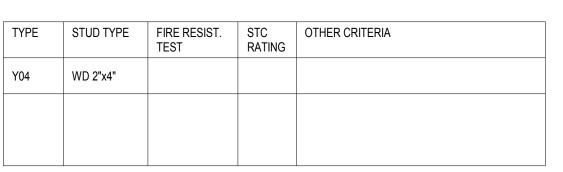


TYPE	STUD TYPE	FIRE RESIST. TEST	STC RATING	OTHER CRITERIA
A14	WD 2"x4"	U465		3-1/2" UNFACED GLASS FIBER SOUND ATTENUATION BATT INSULATION
A14	WD 2"x4"	U465	MIN. 50	3-1/2" UNFACED GLASS FIBER SOUND ATTENUATION BATT INSULATION; ADDITIONAL LAYER OF 5/8" GWB ON EACH SIDE

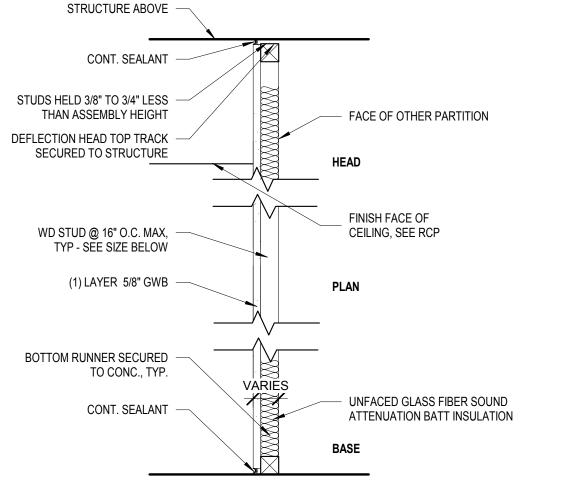
AX4 - RATED WD STUD, DRYWALL PARTITION
1 1/2" = 1'-0"

STRUCTURE ABOVE

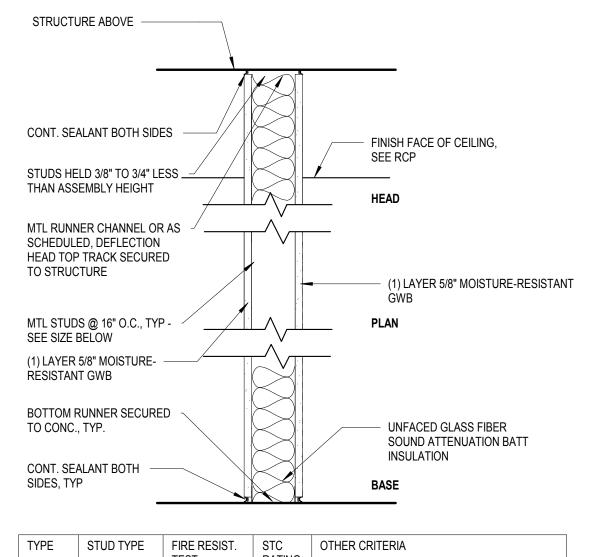




7D Y0X - WD STUD, WD PANEL PARTITION 1 1/2" = 1'-0"

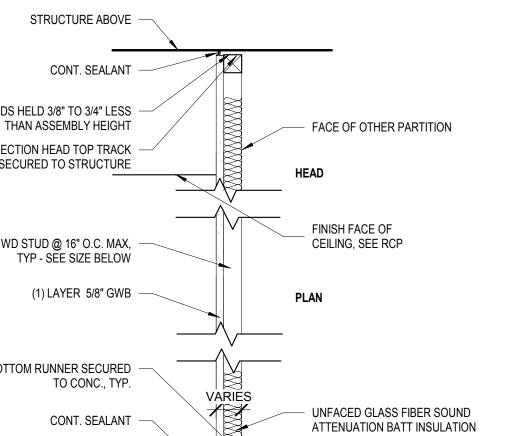


TYPE	STUD TYPE	FIRE RESIST. TEST	STC RATING	OTHER CRITERIA
F02	3/4"			
F04	3 1/2"			3-1/2" UNFACED GLASS FIBER SOUND ATTENUATION BATT INSULATION
F06	5 1/2"			5-1/2" UNFACED GLASS FIBER SOUND ATTENUATION BATT INSULATION



TYPE	STUD TYPE	FIRE RESIST. TEST	STC RATING	OTHER CRITERIA
T01	3/4"			FURRING PARTITION. (1) LAYER 5/8" MOISTURE- RESISTANT GWB ON WET ROOM SIDE.
T04	3 1/2"			(1) LAYER 5/8" MOISTURE-RESISTANT GWB ON WET ROOM SIDES, (1) LAYER 5/8' STANDARD GWB ON NON-WET ROOMS. 3-1/2" UNFACED GLASS FIBER SOUND ATTENUATION BATT INSULATION
T04A	3 1/2"			FURRING PARTITION. (1) LAYER 5/8" MOISTURE- RESISTANT GWB ON WET ROOM SIDE.
T08	7 1/4"			(1) LAYER 5/8" MOISTURE-RESISTANT GWB ON WET ROOM SIDES; (1) LAYER 5/7" STANDARD GWB ON NON-WET ROOM SIDES. 6-1/4" UNFACED GLASS FIBER SOUND ATTENUATION BATT INSULATION

100 TOX - WD STUD, MOISTURE-RESISTANT DRYWALL PARTITION 1 1/2" = 1'-0"



11

PΕ	STUD TYPE	FIRE RESIST. TEST	STC RATING	OTHER CRITERIA
2	3/4"			
4	3 1/2"			3-1/2" UNFACED GLASS FIBER SOUND ATTENUATION BATT INSULATION
6	5 1/2"			5-1/2" UNFACED GLASS FIBER SOUND ATTENUATION BATT INSULATION

100 F0X - FURRING PARTITION 1 1/2" = 1'-0"

TYPE	STUD TYPE	FIRE RESIST. TEST	STC RATING	OTHER CRITERIA
T01	3/4"			FURRING PARTITION. (1) LAYER 5/8" MOISTURE- RESISTANT GWB ON WET ROOM SIDE.
T04	3 1/2"			(1) LAYER 5/8" MOISTURE-RESISTANT GWB ON WET ROOM SIDES, (1) LAYER 5/8' STANDARD GWB ON NON-WET ROOMS. 3-1/2" UNFACED GLASS FIBER SOUND ATTENUATION BATT INSULATION
T04A	3 1/2"			FURRING PARTITION. (1) LAYER 5/8" MOISTURE- RESISTANT GWB ON WET ROOM SIDE.
T08	7 1/4"			(1) LAYER 5/8" MOISTURE-RESISTANT GWB ON WET ROOM SIDES; (1) LAYER 5/7" STANDARD GWB ON NON-WET ROOM SIDES. 6-1/4" UNFACED GLASS FIBER SOUND ATTENUATION BATT INSULATION



PROJECT INFORMATION

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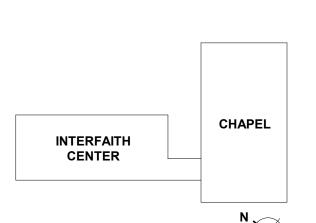
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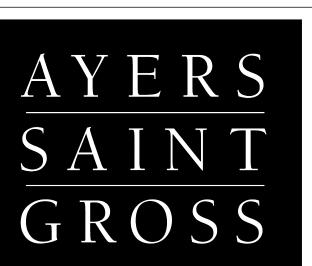
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KEY PLAN





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ISSUE DATE: JOB NO.: DRAWN BY: PROJECT DESIGN PHASE

> **50% CONSTRUCTION DOCUMENTS** DRAWING NAME

PARTITION TYPES

DRAWING NUMBER

Name

1-FLOOR

CPT-1 CARPET TILE

Material

3

Style

MICA TILE, ASHLAR

INSTALLATION

Color

SMOKY QUARTZ

Manufacturer (Basis of

Design)

SHAW

O. Cooci oli lidita yiol (Do			
0.1030			
N - 2 - 3			
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CPT-2 CARPET TILE QUARTZ TILE, ASHLAR SMOKY QUARTZ SHAW 18X36 WORK ROOM ALLOW \$30/SY MATERIAL COST INSTALLATION CPT-3 CARPET TILE INTERFACE HUMAN NATURE, HN 830, BONE 104244 25CM X 1M QUIET LOUNGE BORDER ALLOW \$30/SY MATERIAL COST ASHLAR INSTALLATION CPT-4 CARPET TILE INTERFACE URBAN RETREAT, UR 304, STRAW / BLUE 103659 50CM X 50CM QUIET LOUNGE INSET ALLOW \$40/SY MATERIAL COST BRICK INSTALLAION ALLOW \$40/SY MATERIAL COST CPT-5 CARPET TILE SHAW UNCOVER TILE, 5T150, BURNISHED PEWTER, 50516 9X36 HILLEL BORDER AND OFFICES ASHLAR INSTALLATION INTERFACE CPT-6 CARPET TILE WORLD WOVEN, WW895, HIGHLAND WEAVE, 105377 25CM X 1M HILLEL INSET ASHLAR INSTALLATION ENTRY CPT-7 WALK-OFF MAT TANDUS CENTIVA ASSERTIVE ACTION 04837, LEAD SHOT, 26207 24X24 ALLOW \$20/SY MATERIAL COST BRICK INSTALLATION CPT-8 CARPET TILE DYE LAB, 5T041, BRICK INDIGO, 41496 GREAT ROOM INSET SHAW 24X24 INSTALLATION CPT-9 CARPET TILE INTERFACE HUMAN NATURE, HN 830, COBALT 104240 25CM X 1M LOBBY INSET BORDER ASHLAR INSTALLATION HUMAN NATURE, HN 840. EARTH 104227 CPT-10 CARPET TILE INTERFACE 25CM X 1M LOBBY INSET FIELD ASHLAR INSTALLATION INTERFACE STRAW CPT-11 | CARPET TILE PIN LINE. BRICK 50CM X 50CM PRAYER ROOMS - INSTALLED AT 45 DEGREE ANGLE TO ROOM INSTALLATION CPT-12 BROADLOOM CARPET SHADOW PLUSH 5A127 MUSE 24110 SHAW 12' PRAYER LOBBY PTF-1 PORCELAIN TILE FLOOR MOSAIC TILE SPEAK EASY AV283, 1/3 SWEET GEORGIA BROWN 6X36 LOBBY / GREAT ROOM / KITCHEN / DINING ALLOW \$8/SF MATERIAL COST ASHLAR INSTALLATION PTF-2 PORCELAIN TILE FLOOR ARCHITECTURAL WOOD STOCK, 1/3 6X36 BATHROOMS VANILLA ALLOW \$6/SF MATERIAL COST **CERAMICS** ASHLAR INSTALLATION NOVABELL IMPERIAL HONED RECTIFIED, BRICK CREMA 24X48 ABLUTION AREAS ALLOW \$8/SF MATERIAL COST PTF-3 PORCELAIN TILE FLOOR INSTALLATION SEALED CONCRETE SEALED CONCRETE BUILDING SUPPORT/TELCOM/JANITOR 2-BASE RBB-1 RUBBER BASE **JOHNSONITE** COVE BASE 4" HIGH ROLL BUILDING SUPPORT/TELCOM/JANITOR/OFFICES/CORRIDOR 000A WDB-1 WOOD BASE CHERRY, QUARTER SAWN STAIN COLOR TBD 6" HIGH X 5/8" TH. LOBBY/CORRIDOR 101/GREAT ROOM104/KITCHEN 103/DINING 102, PRAYER ROOMS, PRAYER LOBBY, HILLEL, WORKROOM MATCH EXISTING AT STAIN TO MATCH EXISTING AT CHAPEL WDB-2 WOOD BASE CHAPEL CHAPEL 3-WALL CTW-1 CERAMIC TILE WALL DALTILE **ELEVARE** WHITE 4X16 BATHROOM WALLS CTW-2 CERAMIC TILE WALL NOVABELL IMPERIAL PROVENZALE CREMA 12X12 SHEET OF MOSAICS | ABLUTION AREA WALLS ALLOW \$15/SF MATERIAL COST GTW-1 GLASS TILE WALL TILE BAR KERRIE **KELLY BLUE** 12X12 SHEET OF MOSAICS KITCHEN BACKSPLASH - FULL HEIGHT - REFER TO ELEVATIONS; DINING ROOM ALLOW \$15/SF MATERIAL COST BACKSPLASH PLW-1 PLASTIC LAMINATE WALL PANEL | WILSONART **ELEVATOR CAB** PREMIUM AEON FORBO BULLETIN 2206 OYSTER SHELL TWS-1 TACKABLE WALL SURFACE **LOBBY 101 CORRIDOR BOARD** WDP-1 WOOD PANELING CUSTOM STAIN TO MATCH EXISTING AT CHAPEL MATCH EXISTING AT CHAPEL CHAPEL 4-CEILING APC-1 ACOUSTIC PANEL CEILING **ARMSTRONG** CIRRUS TEGULAR OFFICES, CORRIDOR 000A 24X24 WITH 9/16" GRID APC-2 ACOUSTIC PANEL CEILING **ARMSTRONG** OPTIMA SQUARE 30X60 WITH 9/16" GRID DINING, LOBBY, HILLEL **TEGULAR** MECH / ELEC / CUSTODIAL EXPOSED CEILING PAINTED GYPSUM WALL BOARD PT-2, UNLESS OTHERWISE NOTED 4-MISC RSS-1 RECESSED SOLAR SHADE -LUTRON SHEER WEAVE 2390 - 5% BASKETWEAVE 90 ALL WINDOWS, U.N.O. MANUAL SC-1 SHOWER CURTAIN - @ SHOWER | IINPRO CLICKEZE SHOWER ENCLOSURE SUPER BIO STAT WHITE 5-MILLWORK QTZ-1 QUARTZ COUNTERTOP ZODIAQ CALACATTA NATURA 2CM KITCHEN AND DINING COUNTERTOPS SS-1 HI-MACS 2CM SOLID SURFACE **IVORY WHITE S29** WINDOW SILLS 2CM SS-2 SOLID SURFACE CORIAN GLACIER ICE ABLUTION FLOOR SINKS SOLID SURFACE SS-3 CORIAN 2CM WORK ROOM COUNTERTOP **DUNE PRIMA** CHERRY, QUARTER SAWN STAIN COLOR TBD **VARIES** WD-1 WOOD COLUMN SURROUNDS, CEILING TRIM, WINDOW TRIM, BUILT-IN MILLWORK, LOBBY SCREEN WINDFALL LUMBER CHERRY, SIDE GRAIN STAIN COLOR TBD 1" TH. WDT-1 WOOD TREAD STAIR TREADS WITH INTEGRATED METAL NOSING 6-PAINT P-1 PAINT SHERWIN WILLIAMS | EGGSHELL FINISH SW 7570 EGRET WHITE WALLS THROUGHOUT, UNLESS OTHERWISE NOTED P-2 PAINT SHERWIN WILLIAMS FLAT FINISH SW 7008 ALABASTER GYP BOARD CEILING/SOFFIT THROUGHOUT, UNLESS OTHERWISE NOTED SHERWIN WILLIAMS SEMI-GLOSS FINISH P-3 PAINT SW 6531 INDIGO BATHROOM WALLS SHERWIN WILLIAMS EGGSHELL FINISH P-4 TBD ACCENT WALLS - LOCATION TBD PAINT TBD P-5 PAINT SHERWIN WILLIAMS | EGGSHELL FINISH ACCENT WALLS - LOCATION TBD

5

Size

18X36

FINISH SCHEDULE

OFFICES

6

Location

7

ALLOW \$30/SY MATERIAL COST

9

Remarks

10

11

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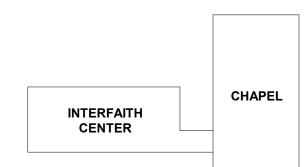
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REVISIONS						
REV. # DESCRIPTION						

KEY PLAN





ARCHITECTS + PLANNERS

NOT FOR CONSTRUCTION

DRAWING INFORMAT	ΓΙΟΝ
ISSUE DATE:	04/28/17
SCALE:	
JOB NO.:	21641.00
DRAWN BY:	ASG
PROJECT DESIGN PH	IASE

PROJECT DESIGN PHASE

50% CONSTRUCTION
DOCUMENTS

DRAWING NAME

FINISH SCHEDULE

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